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1 Supercritical river terraces generated by hydraulic and geomorphic interactions

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- 5

6 ABSTRACT

- 7 The alternating cycle of valley widening through lateral erosion ('strath planation') and valley
- 8 narrowing through vertical incision into bedrock ('strath terrace abandonment') due to
- 9 variations in sediment supply (Q¬s) relative to river transport capacity (Qsc) is a common
- 10 feature in many mountainous environments, yet our understanding of the mechanics of the
- 11 processes that drive this landscape change remains poorly quantified. Here, we used an
- 12 experimental and numerical study to identify the geomorphic and hydraulic controls driving
- the response of mixed bedrock-alluvial rivers to variable sediment supply, water discharge
- 14 and tectonic tilting. The experimental channels exhibit a multi-stage response of channel
- 15 narrowing, stripping of the alluvial cover in a downstream migrating incision wave followed by
- 16 destabilization of the bed and development of a single vertical step in the bed profile
- 17 ('knickpoint') when the hydraulic conditions are supercritical. In our experiments headward
- 18 erosion by knickpoints is the most efficient process of strath terrace abandonment,
- 19 contributing the majority of the total vertical incision in a short period of time. We show
- 20 experimentally, that knickpoint developing under supercritical flow conditions, driving the
- rapid response of fluvial systems to upstream perturbations in Qs/Qsc despite no base-level
- fall. This has implications for the understanding of distributions of strath terrace ages, the
- 23 inference of baselevel variations from knickpoint propagation, and how landscapes respond
- 24 to climatic or tectonic perturbations.

25 INTRODUCTION

- 26 Past cycles of global climatic change have had consequences for the hydrological regime
- 27 and the pattern of sediment production and transport within landscapes (Schumm and
- Parker, 1973; Fuller et al., 2009; Wobus et al., 2010). The balance between the flux of
- 29 sediment entering a mixed bedrock-alluvial river (Qs) and the capacity of the river to
- 30 transport that sediment (Qsc) is important in setting the landscape morphology, such as the
- 31 generation and preservation of flights of strath terraces above the active channel (e.g.,
- 32 Pazzaglia et al., 1998; Wegmann and Pazzaglia, 2002).

Several mechanisms have been proposed for the generation of strath 'terraces', including 33 climate driven oscillations in Qs/Qsc resulting in valley widening (high Qs/Qsc) or vertical 34 incision (low Qs/Qsc) (Bull, 1990; Hancock and Anderson, 2002). The presence of strath 35 36 terraces within a fluvial system have also been shown to result from headward incision by 37 'base-level knickpoints' initiated by fault throw or tectonically driven base level fall (Finnegan, 2013; Finnegan and Balco, 2013), unsteady lateral river erosion (Limaye and Lamb, 2016), 38 39 bedrock structure (Wohl, 2008), the cutoff and migration of river meander bends (Finnegan and Dietrich, 2011), sea level highstands (Merritts et al., 1994), river capture (Stokes et al., 40 2002). The influence of tectonics in generating strath terraces is usually limited to the 41 42 consideration to the initiation of headward migrating incision waves (e.g., Finnegan 2013), 43 while the impact of regional landscape tilting is rarely considered. A clear identification of the mechanism of strath terrace formation is important for the interpretation of the presence and 44 pattern of strath terraces within a landscape to date past climate changes (e.g., Fuller et al., 45 2009; Stokes et al., 2017), estimate fluvial incision rates (e.g., Lavé and Avouac, 2001), or to 46 help calibrate empirical models of fluvial erosion (Finnegan, 2013). However, a universal 47 model of strath terrace formation is currently limited by an incomplete understanding of the 48 mechanics of vertical and lateral erosion in bedrock and difficulties in identifying the erosional 49 pattern during the response of channel processes to perturbations in Qs/Qsc from preserved 50 strath terrace age distributions. 51

52 Here, we present a physical modeling study that explores the mechanisms of channel 53 response and resulting terrace geometry to three scenarios of perturbations to Qs/Qsc. The first two experiments explore the response of a channel following a sudden perturbation to 54 55 Qs/Qsc from the same initial conditions while the third explores the impact of a gradual decrease in Qs/Qsc, through progressive tilting of the experiment. The experiments identify 56 patterns of erosional response including the resulting preserved terrace morphology, aiding 57 the interpretation of terrace age distributions in natural landscapes for purposes such as the 58 reconstruction of climate or tectonic histories (e.g., Fuller et al., 2009; Stokes et al., 2017) 59

60

61 EXPERIMENTAL SETUP

62 Experiments have been used extensively to explore the dynamics of bedrock channels,

- 63 including abrasion processes (e.g., Johnson and Whipple, 2007), the dynamics of bedrock
- 64 plucking (e.g., Dubinski and Wohl, 2013), the mechanics of waterfall erosion in homogenous
- bedrock (e.g., Baynes et al., 2018), and landscape stability (e.g., Hasbargen and Paola,
- 66 2000). We used the 80 cm long, 30 cm wide and 20 cm deep Bedrock River Experimental
- Incision Tank at the Université de Rennes (Fig. 1; S1), with an initial configuration of a thin (2

cm) layer of well-sorted sand (grain size = 250μ m) overlying homogenous silica paste (grain 68 size = 45 μ m). The cohesive silica paste material is commonly used to represent bedrock 69 70 and is eroded by hydraulic shear under clear water flow in the absence of sediment (e.g., Turowski et al., 2005; Turowski et al., 2006; Turowski, 2007) and the experimental channels 71 72 in this study exhibit a similar hydraulic scaling of width and slope to natural bedrock rivers (Baynes et al., 2018). No direct scaling between our experiments and a particular target 73 74 natural system is sought but the behavior of the experiment is similar to that observed in natural systems, allowing the dynamics of processes and their controls to be explored 75 76 quantitatively (Hooke, 1968; SI Section 1).

77 Constant input water discharge, set within 2% by a regulated pump on the inflow pipe, was 78 fed into a reservoir upstream of the inlet, and sediment was added using a Gericke infinite 79 screw feeder system. The channel outlet elevation was fixed, although it was free to migrate across the width of the box, and sediment was collected in a reservoir beyond the outlet to 80 avoid deposition in the channel as a result of the outlet. The same initial conditions for the 81 first two experiments were Q = 1.5 l/min, Qs = 13.33 g/min, slope = 4.5°, selected after 82 83 previous experiments for the best combination of parameters to generate an alluvial river at equilibrium conditions inset in the sand material. Channel slope, width and depth were free to 84 evolve during the course of the experiment through erosion (vertical and lateral) and 85 86 deposition. After 37 minutes, once the channel had reached a constant width, one parameter 87 was altered while all others kept constant to represent a sudden shift in Qs/Qsc; experiment 88 1: Qs reduced to 0 g/min, experiment 2: Q doubled to 3 l/min. In the third experiment, Qs and 89 Q were maintained constant throughout but the slope was increased from an initial slope of 90 2.5° at a continuous rate of 1° per hour for 4.7 hours, gradually decreasing Qs/Qsc. A Leica ScanStation 2 Terrestrial Laser Scanner (green laser footprint = 3 mm) was programmed to 91 collect point clouds of the topography every 2 min. The topography was then gridded with a 92 pixel size of 2 mm, and combined with the hydrodynamic model Floodos (Davy et al., 2017), 93 94 to evaluate the hydraulic conditions within the channel (e.g., Froude number) and to quantify 95 the evolution of the channel geometry (e.g. width and depth) during the experiments (SI 96 Section 2).

97 EXPERIMENTAL RESULTS AND DISCUSSION

Changing Qs/Qsc had a significant, rapid and complex impact on the initial stable state
(channel width: 150 mm) in the first two experiments. Following the removal of input Qs in
experiment 1, an initial narrowing of the channel coincided with a downstream migrating
incision wave initiated at the inlet that removed the alluvial cover (Fig. 2A), exposing the
bedrock surface to erosion. The removal of input Qs led to Qs/Qsc = 0, providing capacity for

the flow to entrain and strip the sediment stored in the alluvial bed. Further narrowing of the 103 channel occurred until the bed destabilized into a series of steps (spaced at 25-30 mm with 104 105 height of 3-5 mm, at t = 83 min) in the bedrock surface that evolved into a single vertical step 106 (15-20 mm in height at t = 111 min) that migrated upstream as a knickpoint, depositing 107 material downstream of the plunge pool (Fig. 2A). The same multi-stage process of 108 downstream stripping of alluvial cover followed by knickpoint development and headward 109 erosion also occurred in experiment 2 where Q was doubled (SI Section 3). The development of steps before the generation of a single knickpoint were not observed in the 110 topographic data from experiment 2 as the channel evolved from downstream stripping to 111 112 headward erosion within the time period between successive laser scans (t = 120 s; Fig. 113 S5B). Despite this, the knickpoint formed mid-channel in the absence of any base level fall or

114 tectonic uplift.

The initial stages of experiment 3 (until t ~100 min) were characterized by an alluvial fan-like 115 116 regime near the inlet with cone-shaped deposits frequently reworked by high lateral channel 117 mobility (Fig. 2B) while the slope increased (Fig. S5C). As Qs/Qsc decreased, the flow focused into a narrower single thread, triggering the onset of a downstream incision wave of 118 the alluvial cover. Subsequently, a knickpoint developed in the bedrock (t = -208 min) which 119 retreated upstream in the same manner as the other experiments (Fig. 2B; SI Section 3). In 120 121 each experiment, the stripping of alluvial cover by the downstream incision wave coincided 122 with a significant narrowing of the channel inset in the alluvial cover, with the narrowest width 123 occurring in bedrock when a knickpoint had formed (Fig. 2B). The channel narrowing 124 associated with the incision of both the alluvial cover and the bedrock led to the 125 abandonment of the formerly active bed at higher elevations, representing the generation of strath terraces seen in natural environments. However, the rates of vertical incision and 126 127 terrace abandonment were not spatially or temporally constant during the experiments, with the highest local rates of vertical incision associated with the downstream stripping of 128 sediment and the headward migration of the knickpoints through bedrock (Fig. 2B; SI Section 129 4). A similar contribution to total vertical incision by knickpoint migration was identified in 130 131 Hawai'i (Mackey et al., 2014).

The Floodos hydrodynamic model output (SI Section 5; Fig. S10) shows that the sequence of steps, and subsequently the knickpoint, developed in experiment 1 where flow had a Froude number > 1. The Froude number, a dimensionless number defined as the ratio of the inertia forces to gravitational forces, characterizes the transition from subcritical to supercritical flow when Fr > 1. During supercritical flow, interactions between the flow and the bed are inherently unstable (Yokokwa et al., 2013; Izumi et al., 2017; Scheingross and Lamb, 2017), and the development of steps in the bedrock surface occur during our experiments when the

- 139 flow conditions exceed the threshold Froude number for supercritical flow. We therefore refer
- to these features as 'supercritical knickpoints' to distinguish them from 'base-level
- 141 knickpoints'. Subsequently, we refer to the terraces generated by the passage of supercritical
- 142 knickpoints as 'supercritical terraces'.

143 IMPLICATIONS FOR THE UNDERSTANDING OF NATURAL LANDSCAPES

Our experiments provide direct evidence of the hydraulic processes that lead to strath 144 terrace formation following perturbations to Qs/Qsc. Wobus et al. (2010) show that climate 145 change induced incision was associated with a downstream incision wave due to changes in 146 147 the relative sediment supply to transport capacity, while base-level induced (e.g, river capture, tectonics) incision was characterized by an upstream migrating incision wave. 148 Crucially, our experiments show that channels in homogenous substrate respond to climatic 149 perturbations through a complex multi-stage incision mechanism, containing a downstream 150 (Fig. 3A) and upstream (Fig. 3C) wave component that both generate terraces, in the 151

- absence of any base level fall.
- 153 The multi-stage nature of the incision (Fig. 3) has important implications for studies that use
- the age of terraces to reconstruct climatic changes (e.g., Fuller et al., 2009) or rates of fluvial
- incision and tectonic uplift (e.g., Lavé and Avouac, 2001), due to the time transgressive
- nature of the terraces generated by this response (Zaprowski et al., 2001). During the
- downstream incision wave, the upstream parts of the alluvium are abandoned first, while the
- 158 opposite is true during the upstream incision wave, leading to spatially variable terrace ages
- 159 (Fig. 3D). If it is incorrectly assumed that the entire length of a terrace was abandoned at the
- same time, the average vertical incision rate calculated from a single terrace abandonment
- age would be misleading due to the failure to identify the erosion history of an incision wave
- and the spatially and temporally non-uniform nature of the river incision (Finnegan et al.,
- 163 2014). Strategic sampling of high-resolution point ages from terraces may differentiate
- between strath terrace formation by vertical incision or knickpoint retreat (e.g., samples
- 165 collected longitudinally from the same terrace surface; Baynes et al., 2015), allowing
- accurate interpretation of the mechanisms and triggering perturbations that generated them.
- 167 Knickpoints in a channel are traditionally interpreted as markers of either local lithological
- boundaries (e.g., Haviv et al., 2010), a history of base level fall or tectonic uplift rate
- variability (e.g., Finnegan, 2013), or threshold drainage areas for channel incision (Crosby
- and Whipple, 2006). However, our experiments indicate that the presence of a knickpoint in a
- 171 channel with homogenous bedrock and constant baselevel is not necessarily diagnostic of
- any of these sources as knickpoints are generated following perturbations to Qs/Qsc (Fig. 3).
- 173 Under normal conditions, flow in large natural alluvial rivers where strath terraces are

commonly found is subcritical, but above-threshold supercritical flow conditions (Fr > 1) can 174 occur due to a combination of driving factors including channel narrowing with variable width 175 176 and increased discharge (SI Section 5). Base-level terraces are likely to have the same or a 177 similar slope to the active channel bed through parallel knickpoint retreat (Finnegan, 2013), 178 whereas terraces generated during the multi-stage response identified here are steeper than the active channel (Fig. 2, 3) due to new equilibrium conditions with lower sediment supply, 179 180 higher discharge or continued tilting. An additional diagnostic link could be the highest terrace having a decreasing abandonment age with distance downstream (Fig. 3D) that does 181 182 not occur during terrace formation driven by base-level fall. Thus, the morphology of terraces 183 and spatial patterns of abandonment ages can be diagnostic of the processes and forcing 184 perturbations that led to terrace generation and should be considered carefully when analyzing topography for the purpose of understanding and reconstructing past tectonic or 185

186 climatic histories.

187 CONCLUSIONS

Landscapes are able to respond rapidly following climatic or anthropogenic perturbations that 188 189 switch the erosion processes from one regime to another. The transition from lateral erosion 190 and strath planation to channel narrowing, incision and strath terrace abandonment occurs 191 through a multi-stage process of downstream then upstream incision waves. The headward 192 passage of knickpoints can be initiated even in the absence of base level fall, due to the 193 interaction between the channel bed and supercritical flow conditions. The passage of supercritical knickpoints provide an efficient process for the abandonment of strath terraces, 194 and should be considered when using the presence of strath terraces within landscapes to 195 infer past fluvial incision rates or climatic changes, which may be more spatially and 196 197 temporally variable than previously thought.

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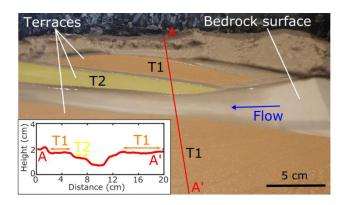
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- 318

319 FIGURES



320

321 Figure 1. : Example of terraces generated in the experiments. The white surface is the silica

322 paste acting as cohesive bedrock and the brown material is sand, acting as coarse material

- 323 transported as bedload. Progressive channel incision and channel narrowing leads to the
- 324 abandonment of the previously active river bed, preserved as strath terraces. Inset shows
- 325 topographic cross-section from A to A', with terraces labeled in yellow and orange.

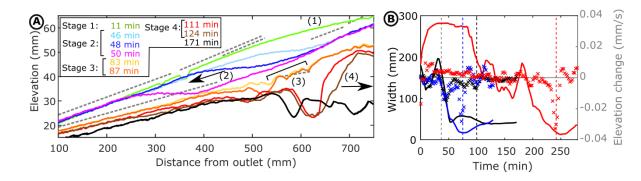
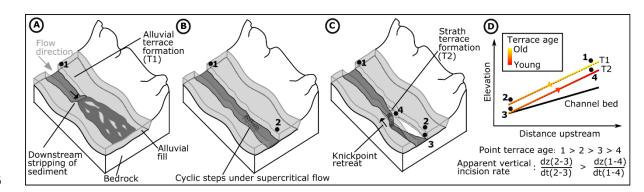


Figure 2. (A). Extracted long profiles during experiment 1. Green line (t = 11 min) shows the 327 profile of the alluvial river, with the subsequent stripping of the sediment cover progressively 328 exposing the bedrock surface within the channel. Black solid line (t = 171 min) shows the 329 final long profile. Grey dashed lines indicate the location of preserved terraces at the end of 330 331 the experiment. Instabilities form mid-channel, developing into a single knickpoint step that 332 propagates upstream. (1): Initial alluvial channel. (2): Downstream stripping of alluvial cover. (3): Cyclic step development. (4): Knickpoint retreat in headward direction. (B): Evolution of 333 334 channel width (lines) and rate of vertical elevation change (crosses) through time for the 335 experiments (expt 1 = black, expt 2 = blue, expt 3 = red) at a cross-section 25% distance 336 from the channel inlet. Width is plotted using a 10 point moving average and elevation change is plotted using a 5 point moving average. Dashed vertical lines indicates the timing 337 of the perturbation in experiments 1 and 2 at 37 mins (gray), and the timing of the presence 338 339 of a knickpoint (colored lines). After the perturbation, there was a reduction in channel width during both experiments 1 and 2. Focusing of the flow into a single thread occurred due to 340 the gradual tilting in experiment 3. There was a short period of channel widening between 341 342 150 and 175 min associated with the erosion of the sediment deposited at the inlet during the initial stages, unrestricting the flow and inundating a larger area and an apparent increase in 343 the channel width. The highest incision rates are associated with (i) the rapid downstream 344 stripping of sediment and (ii) the headward migration of a knickpoint through bedrock. 345



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347 Figure 3. Conceptual model showing the stages of supercritical terrace formation. (A)

348 Downstream stripping of sediment cover following perturbation to Qs/Qsc. (B) Under

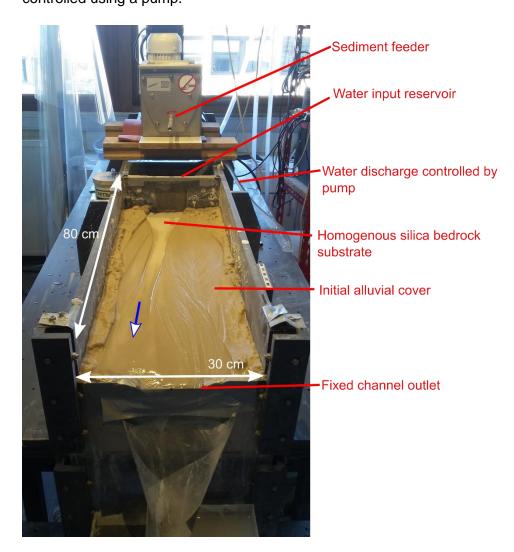
- 349 supercritical flow conditions, the channel bed destabilizes into series of cyclic steps. (C)
- 350 Cyclic steps develop into a 'supercritical knickpoint', which migrates in an upstream direction
- abandoning a 'supercritical river terrace'. Channel narrows at knickpoint, and widens
- downstream. (D) Conceptual representation of the elevation of the channel bed and
- 353 corresponding terraces following response shown in (A-C). Arrows on terraces indicate the
- direction of the incision wave that abandoned them. The upstream part of T1 is abandoned
- 355 first by the downstream migrating incision wave, and the downstream part of T2 is
- abandoned first by the upstream migrating knickpoint. Vertical incision rates calculated from
- 357 point terraces ages (1–4 and 2–2) provide misleading spatial variation in vertical incision, as
- 358 the majority of the incision actually occurs through the passage of the supercritical
- 359 knickpoint.

361 Supplementary Information

362 SI Section 1: Relevance of experiments for natural channels and experimental setup.

363 Experimental modelling studies of landscape and fluvial dynamics are typically performed 364 using one of two approaches. The first directly scales the experimental prototype to the 365 natural system by ensuring that non-dimensional hydraulic parameters such as the Froude number are scaled strictly and the physical processes (e.g., sediment transport) are the 366 same in both the experimental and natural systems. The second approach relaxes this strict 367 scaling, and seeks a 'similarity of process' between the experiment and the natural channel 368 369 (Hooke, 1968) whereby the behaviour of the processes within the experiment system are 370 qualitatively similar and 'reasonably effective' at replicating the behaviour of natural systems 371 despite large differences in the governing dimensionless numbers (Paola et al., 2009). This second approach has the benefit of allowing temporal and spatial timescales to be relaxed, 372 as materials such as silica cement that is eroded by clear water flow can be used to replicate 373 374 bedrock, and experiments exploring bedrock erosion processes can be performed relatively guickly. It should be noted that the results from these experiments cannot be directly scaled 375 376 up to any particular natural system. Additionally, the experimental channel is always 'active', and the experiments do not include the periods of time between flood events when the 377 378 channel is not being eroded, creating a difficulty when exploring relative timescales of 379 geomorphic response between the experiments and natural rivers. However, processes such 380 as flow acceleration above the knickpoint lip, cyclic step development, plunge pool erosion, undercutting of the knickpoint face and channel banks, cantilever failure, erosion and 381 transport of cohesive material by hydraulic shear are present in the experiments. The typical 382 width-depth ratio of the experimental channels at the start of the experiments (alluvial 383 384 channel conditions) is 50-80, similar to that of natural gravel-bedded rivers (Finnegan et al., 2005), and the scaling of the channel width with discharge observed in natural rivers is also 385 reproduced in the experiment. Therefore, we can be confident that the experiments are 386 'reasonably effective' in replicating natural processes, and the response of the channels to 387 388 the perturbations (sediment supply, discharge or slope) in the experiments can help inform the response of natural channels to similar forcing. 389

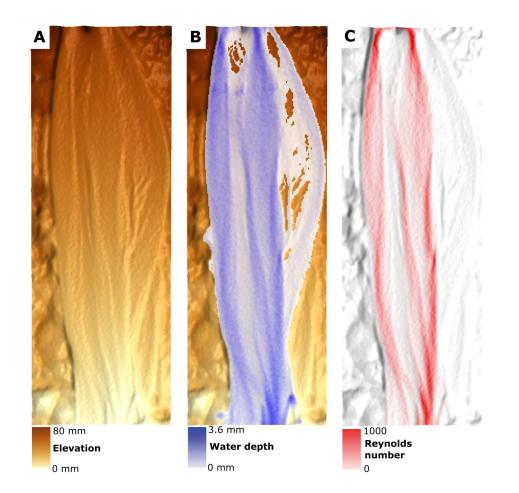
Figure S1 shows the setup used during our experiments in the Bedrock River Experimental Incision Tank at the Université de Rennes. The initial alluvial cover was composed of wellsorted sand (grain size = 250μ m), and the homogenous cohesive silica paste used as a bedrock simulant was composed of 61.5% angular silica, 20.5% spherical beads and 18% water. The silica paste was mixed in a cement mixer before being transferred into the box flume and homogenised using a high frequency vibrating needle to re-liquefy the paste (see Baynes et al., 2018 for further details related to the properties of the silica paste and the preparation procedure). Flow in the channel is typically laminar with low Reynolds numbers (< 1000), and is input into the channel through a reservoir and the discharge magnitude controlled using a pump.



400

- Fig. S1: Photograph of the Bedrock River Experimental Incision Tank at Université de
 Rennes. The blue arrow shows the flow direction within the channel.
- 403 SI Section 2: Floodos parameters

Floodos is a numerical precipiton-based hydrodynamic model (see Davy et al., 2017 for full
technical description of the model), and was used here to assess the hydraulic conditions
during the experiments. The input topography for *Floodos* was produced directly by gridding
the point clouds, collected from the terrestrial laser scanner at regular time intervals during
the experiments, with a pixel size of 2 mm. *Floodos* was run using the laminar flow option
consistent with the relatively low Reynolds number of the experiment (< 1000) across most of
the channel under alluvial conditions (Fig. S2).



412 **Fig. S2:** (A) Digital Elevation Model from Experiment 1, when the channel is under fully

413 alluvial conditions (t = 33 minutes). (B) Modelled water depth mask using Floodos

414 hydrodynamic model (Davy et al., 2017). (C) Calculated Reynolds number for each pixel of

415 the flow, using the Floodos model output.

416

411

417 Under the laminar flow conditions, the local vertically averaged velocity *U* is approximated 418 by:

419

 $U = CH^2 S, \tag{S1}$

where C if a friction coefficient in [L⁻¹.T⁻¹], H is local water depth and S is the local channel 420 slope. Equation S1 is obtained by integrating vertically a laminar flow velocity profile 421 assuming steady-uniform flow. In pure laminar flow, C is entirely set by the water viscosity μ 422 423 and should be approximately $C \sim \rho g/3\mu$ where ρ is water density and g is gravitational acceleration. This predicts that at 10°c, C ~ 2.5 x 10⁶ m⁻¹.s⁻¹. Fig. S2 shows the impact of 424 425 different values of C on the predicted extent of water flowing over a flat surface compared to manual measurements of the wetted area obtained from a photograph taken from above the 426 experiment (Fig. S3A). The simulations of flow extent are relatively similar for all modelled 427

- 428 values of C, although lower values of C predict a greater level of overflow on the edges in the
- downstream part. We therefore used the value of C ($2.5 \times 10^6 \text{ m}^{-1}.\text{s}^{-1}$) for the simulations
- 430 presented here, due to good consistency with the manual measurements and the theoretical
- 431 prediction of the friction coefficient based on the water viscosity for flow over a flat surface
- 432 (Fig. S3) and for channelised flow (Fig. S4).
- 433

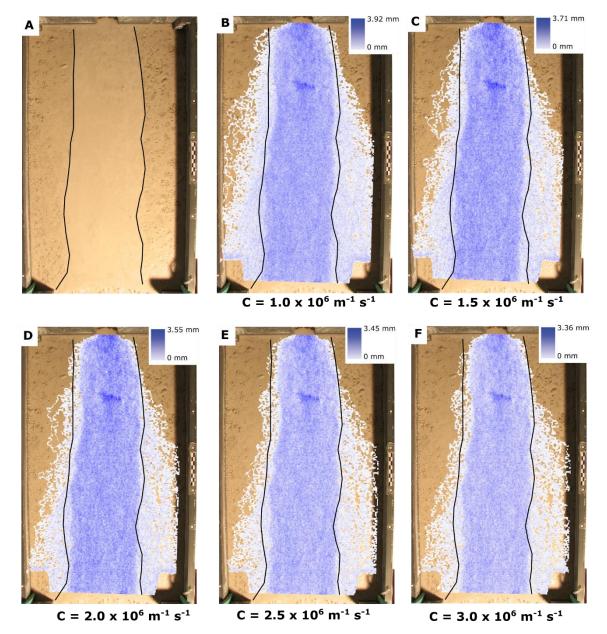


Fig. S3: (A) Photograph of water flowing over a flat surface (slope = 3.3°, Q = 1 l/min), with black lines indicating edge of water extent measured manually from the image. Flow is from top to bottom in the image. (B-F) Floodos hydrodynamic model output using laminar flow option and different values of the friction coefficient. Black lines are the same as (A) and shown for comparison of modelled against observed flow extent. The predicted theoretical

- 440 value of the friction coefficient $(2.5 \times 10^6 \text{ m}^{-1} \text{ s}^{-1})$ has a good match with the observed flow
- 441 extent. For this value, the lateral areas where overflow occurs have flow depth that are
- extremely low (< 0.1 mm) and represent a negligible fraction of the total discharge.

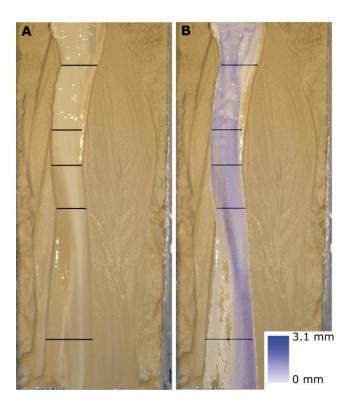


Fig. S4: (A) Photograph of experiment 1 (t = 66 mins), with wetted area indicated by the
black bars. Flow is from top to bottom in the image. (B) Floodos model water mask coloured

by water depth, using friction coefficient of $2.5 \times 10^6 \text{ m}^{-1} \text{ s}^{-1}$, for the same time period overlain

447 over the photograph, with the same black bars from (A). The Floodos water mask closely

448 matches the manual measurements of channel width from the photograph throughout the

length of the channel. At the left hand side in the downstream reach, a terrace is starting to

450 be abandoned, hence the very low flow depths in this region.

451

452 SI Section 3: Evolution of channel and terrace geometry during experiments

This SI section provides the complete experimental data for the evolution of the channel geometry for each of the three experiments. The long profiles were extracted by finding the point of minimum elevation for each row of the grid (Fig. S5). Terraces were defined from the DEM as flat surfaces (elevation within 1mm) and at least 2 cm wide. The channel crosssections (Fig. S6) were extracted from the data at the location of the grid that was 25% of the distance from the inlet to the outlet of the channel.

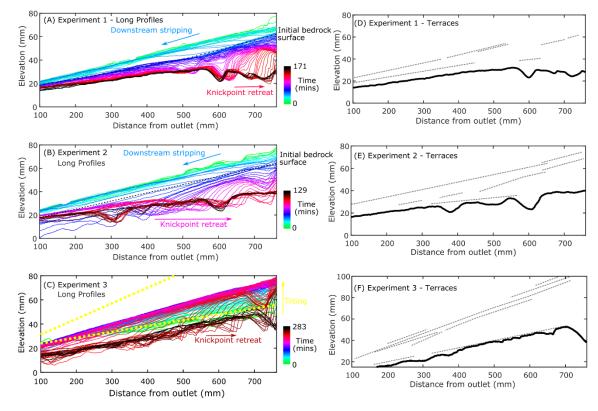




Fig. S5: (A-C) Extracted long profiles from the three experiments. (A) Experiment 1, where 461 462 the input sediment flux was stopped after 37 minutes (80 profiles in total). (B) Experiment 2, where the discharge was doubled to 3 l/min after 37 minutes (61 profiles in total). (C) 463 Experiment 3, where the slope was increased at 1° per 60 minutes from an initial slope of 2° 464 for 280 minutes (132 profiles in total). The colour of the lines indicates the evolution during 465 the experiments. The dashed yellow lines in (C) show the initial slope of the experiment 466 (2.5°) , and the projected slope of the channel at the end of the tilting if there had been no 467 erosion of the alluvial cover or bedrock (7.2°). The maximum slope achieved in the channel 468 before the onset of the incision of bedrock was $\sim 4.5^{\circ}$. Grey dashed lines in (A) and (B) 469 470 indicate the elevation of the bedrock surface at the start of each experiment, to show the transition from alluvial stripping and bedrock incision. The same line is not shown in (C) as 471 472 the elevation of the bedrock surface changed during the course of the experiment due to the tilting. (D-F) Preserved terrace surfaces at the end of each experiment. Solid lines indicate 473 the long profile of the active channel, and dashed lines indicate the location and elevation of 474 terraces. The slope of the upper terraces is greater than the incised active channel. 475

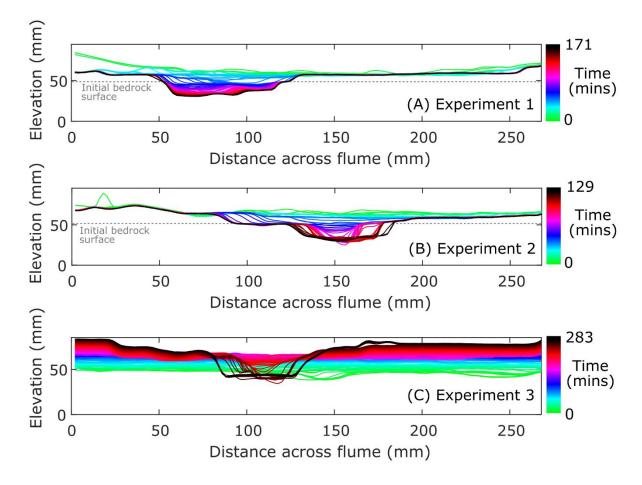


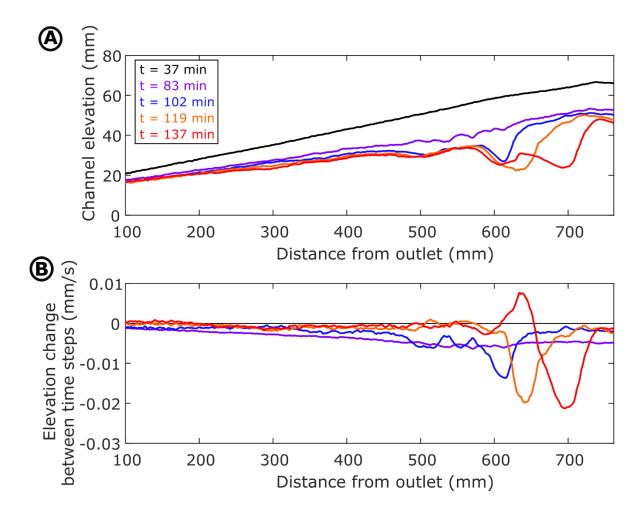
Fig. S6: Evolution of channel cross-sections at the 25% position from the water inlet. Cross-478 479 sections were extracted from this location to ensure it was located over an area that 480 experienced incision and the development and propagation of a knickpoint. Colours represent the same time points as Fig. S1. It can be seen that channel incision occurred 481 under narrow channel width conditions, followed by lateral erosion of the banks leading to 482 483 channel widening and minor channel aggradation in experiments 2 and 3. Grey dashed lines in (A) and (B) indicate the elevation of the bedrock surface at the start of each experiment, to 484 485 show the transition from alluvial stripping and bedrock incision. The same line is not shown in 486 (C) as the elevation of the bedrock surface changed during the course of the experiment due 487 to the tilting.

477

SI Section 4: Demonstration of the incisional efficiency of the headward erosion process and width-incision coupling

- 491 Here, we provide further analysis to demonstrate the bedrock erosional efficiency of the
- 492 headward migration of knickpoints compared to the vertical erosion by hydraulic shear
- 493 upstream of the knickpoint lip (Fig. S7). Fig. S7A shows selected long profiles from
- 494 experiment 1 and Fig. S8B shows the rate of elevation change along the profiles between the

time steps. The largest rates of bedrock elevation change correspond to the locations andtime periods when the knickpoint is migrating upstream.



497

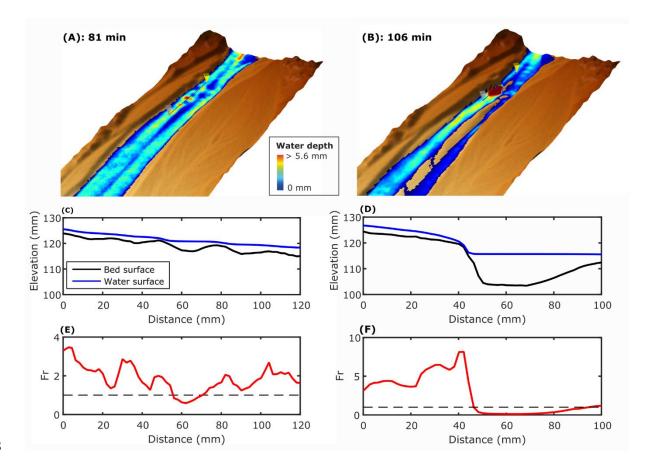
Fig. S7: (A) Selected long profiles showing alluvial channel just before sediment supply 498 stopped (t = 37 min; black), channel containing cyclic steps, after sediment has been stripped 499 500 away from the bedrock profile (t = 83 min; purple) and then three long profiles from different 501 stages of knickpoint retreat (t = 102, 119, 137 min; blue, orange and red, respectively). (B) 502 The variation in rate of elevation change with distance along the channel between the 503 different time steps. The consistently high rate of elevation change from t = 37 minutes to t =504 83 minutes (purple line) corresponds to the time period when the alluvial cover was being 505 stripped downstream.

506

507 SI Section 5:

508 Conditions for cyclic step and knickpoint development

- 509 'Supercritical knickpoints' were generated during the experiments in the absence of any base
- 510 level fall, after the development of instabilities in the channel bed possibly associated with
- supercritical flow conditions, when the Froude number ($Fr = U/(gH)^{0.5}$), is > 1 (Fig. S9; Parker
- and Izumi, 2000). In this section, we used the Floodos hydrodynamic model output to
- calculate the Froude number for all locations within the channel (Fig. S8) for selected times
- of experiment 1 (t = 81 min, when bed instabilities were developing; Fig. S8A, and t = 106
- 515 min, when a single knickpoint had formed; Fig. S8B).
- 516 Cyclic steps form under supercritical flow conditions due to an interaction between the
- 517 channel bed and the flow above it (Izumi et al., 2017), developing a series of steps with
- transcritical flow that alternates between supercritical and subcritical flow with hydraulic
- 519 jumps (Fig. S8A). After a single knickpoint formed, a pronounced change in the Froude
- number and a hydraulic jump is present at the knickpoint lip, with subcritical flow conditions
- 521 present in the plunge pool downstream (Fig. S8B). The absolute values of Fr are possibly
- 522 slightly overestimated due to the use of the laminar flow option in *Floodos*, and some
- 523 uncertainty related to whether the flow is purely laminar or transitional (Fig. S2). However,
- 524 the pattern of Fr variation is consistent with cyclic step development as observed and
- 525 predicted theoretically under supercritical flow conditions (Parker and Izumi, 2000; Izumi et
- 526 al., 2017).



528

Fig. S8: Exploration of flow conditions during the development of the self-formed knickpoints.
(A-B) Topography during the experiments at selected time steps, with the colours indicating
the water depth. (C-D) Profiles for the bed and water surfaces for the reach along the black
lines in (A) and (B). (E-F) Extracted Froude number along the same reach as the (C) and (D).

534 Supercritical flow in natural rivers

Here, we performed a simple test to consider whether supercritical flow conditions can be

sustained in natural rivers under steady, uniform flow, by exploring the impact of varying

channel width and discharge on the Froude number, assuming a rectangular channel with aconstant slope.

The flow velocity can be estimated using the Manning's equation ($U = n^{-1}.H^{2/3}.S^{0.5}$), where n is the Manning's roughness coefficient, H is the flow depth, and S is the channel slope. The flow velocity is also given by U = Q / WH, where Q is the discharge and W is the channel

width. Substituting these two equations into each other gives the following equation for theflow depth:

544
$$H = \left(n \frac{Q}{W}\right)^{\frac{3}{5}} S^{-\frac{3}{10}}$$
 (S2)

The Froude number is calculated from the flow velocity, the acceleration due to gravity (g) and the flow depth: ($Fr = U/(gH)^{0.5}$). Substituting equation S2 into this equation gives the following relationship between the Froude number and the discharge, slope, width and Manning's roughness coefficient:

549
$$Fr = Q^{\frac{1}{10}} S^{\frac{9}{20}} W^{-\frac{1}{10}} n^{-\frac{9}{10}} g^{-\frac{1}{2}}$$
 (S3)

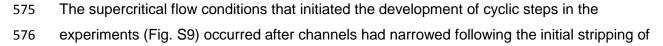
550 Eq (S3) shows that in steady uniform flow conditions, the Froude number is mostly sensitive 551 to slope and roughness, with a weak dependency on width and discharge.

552 According to Parker et al., (2007), the channel width and channel slope of a gravel bed river 553 at bankfull discharge conditions can be calculated using the following equations:

554
$$W_{bf} = \frac{4.63}{g^{\frac{1}{5}}} Q_{bf}^{0.4} \left(\frac{Q_{bf}}{\sqrt{gD_{50}} D_{50}^2} \right)^{0.0667}$$
 (S4)

555
$$S_{bf} = 0.101 \left(\frac{Q_{bf}}{\sqrt{gD_{50}}D_{50}^2}\right)^{-0.344}$$
 (S5)

Where Q_{bf} is the bankfull discharge, W_{bf} is the channel width at bankfull conditions, S_{bf} is the 556 557 channel slope at bankfull conditions and D₅₀ is the median grain size. Using these 558 relationships, we calculated the Froude number for bankfull conditions at a range of 559 discharges in an idealized gravel bed river, assuming a median grain size of 10 cm and two values of the Manning's roughness coefficient. The Froude number at equilibrium conditions 560 using the relationships proposed by Parker et al., (2007) are subcritical, typically in the range 561 0.2-0.3, whatever the value of the bankfull discharge. This shows that in steady uniform flow 562 conditions, single thread gravel bed rivers are not expected to be close to supercriticality, 563 and would not do so even after significant narrowing due to the weak dependency of Fr on 564 channel width (eq. S3). The Parker et al., (2007) relationships (equations S4, S5) are 565 empirical based on a dataset of 72 gravel bed rivers in the UK, Canada, and the USA, which 566 exhibit a degree of universality between them, but do not pertain to braided rivers which are 567 very often the state of rivers during terrace aggradation phases (e.g., Hanson et al., 2006). 568 Using high resolution lidar topography and the Floodos hydrodynamic model, we explored 569 570 the characteristics of the flow conditions at bankfull conditions for a braided river, the Rakaia river in New Zealand. The present day conditions of the Rakaia are a good analogue both for 571 the initial conditions of the experiments and for the former conditions of many present day 572 573 rivers that are incised with flights of terraces abandoned on the valley sides (e.g., Charwell 574 River, New Zealand; Bull, 1990).



- 577 the sediment cover. Therefore, we performed some simple tests using the DEM of the
- 578 Rakaia river to explore the impact of different constant channel widths with a fixed discharge
- 579 on the Froude number (Fig. S9A), and the impact of different discharges in a constrained
- 580 fixed width channel (Fig. S9B). For a fixed discharge, a narrower channel increases the
- average value of the Froude number and for a fixed width, increased discharge also
- 582 increases the average values of the Froude number.

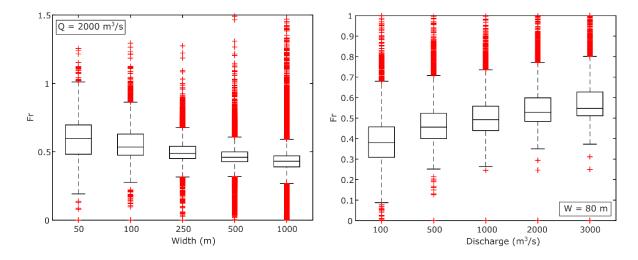
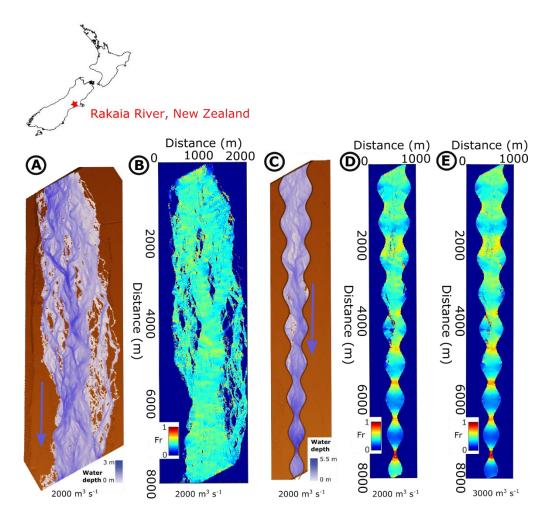


Fig. S9: (A) Impact of channel width on Froude number for a fixed discharge ($Q = 2000 \text{ m}^3 \text{ s}^-$ ¹). (B) Impact of discharge on Froude number for a fixed width (W = 80 m). As predicted by equation S3, both width and discharge have a small impact on the average Froude number in the channel, but the flow can become supercritical in a few locations.

583

589 The average values of the Froude number remain sub-critical for the scenarios presented in 590 Fig. S9, but the peak values of Fr approach supercriticality due to the local spatial variations 591 in the flow conditions. The numerical simulations in Fig. S11 were carried out using a straight 592 channel, which is overly simplistic for a natural river system. The maps of Froude number for the Rakaia (Fig. S10B) river indicate that flow conditions are not spatially uniform at bankfull 593 discharge and are subcritical (Fr < 1) but the values of the Froude number can be higher 594 (~0.4-0.6) than the values predicted by the relationships of Parker et al., (2007). However, it 595 596 is possible to have supercritical flow conditions during frequent floods when the flow is focussed into a narrower channel with a spatially non-uniform channel width (Fig. S10C-E). 597 598 Such conditions may be present in natural settings following a perturbation to the sediment supply or transport capacity and the subsequent stripping of sediment cover and focussing of 599 600 the flow into a significantly narrower channel with spatially variable width (Fig. 2A). The 601 simulations in Fig. S10 were carried out with no alteration to the channel slope. If the slope of 602 the bedrock surface is steeper than the slope of the alluvial channel flowing over the

- sedimentary deposits (e.g. Finnegan and Balco, 2013), the likelihood of supercritical flow and
 the possible development of bed instabilities that evolve into knickpoints is increased. We
 therefore suggest that the processes leading to rapid strath terrace generation through selfformed knickpoint retreat, in the absence of any vertical perturbation to the bed elevation,
 identified in the experiments are possible in natural environments.
- 608
- 609



611 Fig. S10. (A) Map of water depth for a reach of the Rakaia river, New Zealand, under

- bankfull conditions (Q = 2000 $m^3 s^{-1}$), modelled using Floodos on a Lidar generated DEM
- 613 (Released under Creative Commons By Creative Commons Attribution 3.0 New Zealand
- 614 Link: <u>http://data.linz.govt.nz/license/attribution-3-0-new-zealand/;</u> downloaded from
- 615 <u>www.opentopography.org</u>; DOI: 10.5069/G9JQ0XZV). (B) Map of Froude number, typically
- ~0.4, with some local areas slightly higher at ~0.6. Channel width variability and degree of
- 617 channel constriction has impact on the water depth (C) and Froude number at $Q = 2000 \text{ m}^3 \text{ s}^-$
- 618 ¹ (**D**) and $Q = 3000 \text{ m}^3 \text{ s}^{-1}$ (**E**), with increasing degree of supercriticality with increased
- 619 channel constriction and increased discharge.

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