

# GPR-inferred fracture aperture widening in response to a high-pressure tracer injection test at the Äspö Hard Rock Laboratory, Sweden

Justine Molron, Niklas Linde, Philippe Davy, Ludovic Baron, Caroline Darcel, Jan-Olof Selroos, Tanguy Le Borgne, Diane Doolaeghe

# ▶ To cite this version:

Justine Molron, Niklas Linde, Philippe Davy, Ludovic Baron, Caroline Darcel, et al.. GPR-inferred fracture aperture widening in response to a high-pressure tracer injection test at the Äspö Hard Rock Laboratory, Sweden. Engineering Geology, 2021, 292, pp.106249. 10.1016/j.enggeo.2021.106249. insu-03274403

# HAL Id: insu-03274403 https://insu.hal.science/insu-03274403

Submitted on 30 Jun 2021

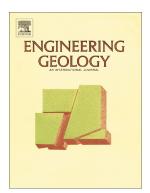
**HAL** is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers.

L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.



Distributed under a Creative Commons Attribution - NonCommercial - NoDerivatives 4.0 International License

GPR-inferred fracture aperture widening in response to a highpressure tracer injection test at the Äspö Hard Rock Laboratory, Sweden



Justine Molron, Niklas Linde, Philippe Davy, Ludovic Baron, Caroline Darcel, Jan-Olof Selroos, Tanguy Le Borgne, Diane Doolaeghe

PII:	S0013-7952(21)00260-X
DOI:	https://doi.org/10.1016/j.enggeo.2021.106249
Reference:	ENGEO 106249
Τ	
To appear in:	Engineering Geology
Received date:	21 January 2021
Revised date:	25 May 2021
Accepted date:	25 June 2021

Please cite this article as: J. Molron, N. Linde, P. Davy, et al., GPR-inferred fracture aperture widening in response to a high-pressure tracer injection test at the Äspö Hard Rock Laboratory, Sweden, *Engineering Geology* (2021), https://doi.org/10.1016/j.enggeo.2021.106249

This is a PDF file of an article that has undergone enhancements after acceptance, such as the addition of a cover page and metadata, and formatting for readability, but it is not yet the definitive version of record. This version will undergo additional copyediting, typesetting and review before it is published in its final form, but we are providing this version to give early visibility of the article. Please note that, during the production process, errors may be discovered which could affect the content, and all legal disclaimers that apply to the journal pertain.

© 2021 The Author(s). Published by Elsevier B.V.

### GPR-inferred fracture aperture widening in response to a high-pressure

## tracer injection test at the Äspö Hard Rock Laboratory, Sweden

Intended for publication in Engineering Geology

Justine MOLRON<sup>1, 3\*</sup>, Niklas LINDE<sup>2</sup>, Philippe DAVY<sup>3</sup>, Ludovic BARON<sup>2</sup>, Caroline DARCEL<sup>1</sup>, Jan-Olof

SELROOS<sup>4</sup>, Tanguy LE BORGNE<sup>3</sup>, Diane DOOLAEGHE<sup>1, 3</sup>

<sup>1</sup> Itasca Consultants S.A.S., 29 Avenue Joannès Masset, Lyon 69009, France j.molron@itasca.fr\*, d.doolaeghe@itasca.fr & c.darcel@itasca.fr

<sup>2</sup> Institute of Earth Sciences, University of Lausanne, Géopolis UNIL, Quar ier N puline, 1015 Lausanne, Switzerland. niklas.linde@unil.ch & ludovic.baron@unil.ch

<sup>3</sup> Univ Rennes, CNRS, Géosciences Rennes, UMR 6118, 263 Avenue Geuréral Leclerc, Rennes 35042, France. tanguy.le-borgne@univ-rennes1.fr & philippe.davy@univ-rennes1.fr

<sup>4</sup> Swedish Nuclear Fuel and Waste Management Company (SKL), Evenemangsgatan 13, Box 3091, SE-169 03 Solna, Sweden.

jan-olof.selroos@skb.se

### 1 Introduction

Characterization of flow and transport in fractured media presents important challenges. Flow in fractures is controlled by fracture aperture, fracture geometry and the connectivity between fractures forming a network (Gudmundsson, 2011). The high variability of these structural parameters results in complex preferential flow paths (Day-Lewis et al., 2017; Shakas et al., 2016; Tsang & Neretnieks, 1998; Tsang & Tsang, 1989; Tsoflias & Becker, 2008) with channelized transport (Klepikova et al., 2016; National Research Council, 1996) that enhances contaminant transport and makes remediation of contaminated sites extremely difficult (N. 'ional Research Council, 1996; Neuman, 2005). The ability to obtain quantitative data, in situ, or flow paths and on hydraulic and hydromechanical properties is a prerequisite to understand such systems and to develop reliable predictive models (Selroos et al., 2002). The combined use of hydraulic tests and geophysical monitoring may offer such data at appropriate scares (Day-Lewis et al., 2017). In this paper, we carried out a tracer experiment with GPR monitoring in a low-permeability formation at depth to detect and quantify the spatial and hydraulic properties of a fractured system. An originality of this study resides in an attempt to quanti'y the evolution of the hydraulic properties under injection pressure in relation to the hydro nechanical response of these fractured geological formations (Rutqvist et al., 1998; Zang et al., 20.5).

Time-lapse ground penetrating radar (GPR) data can offer dynamic imaging, at high spatial resolution, of tracer transport in fractured media (Becker & Tsoflias, 2010; Day-Lewis et al., 2003; Dorn et al., 2011a; Dorn et al., 2011b; Giertzuch et al., 2020; Shakas et al., 2016; Shakas et al., 2020; Talley et al., 2005; Tsoflias & Becker, 2008). The antenna frequency affects the sensitivity to fluid conductivity (Day-Lewis et al., 2003; Talley et al., 2005; Tsoflias et al., 2001) and fracture aperture (Tsoflias & Becker, 2008). Low to intermediate frequencies (25-200 MHz) have high sensitivities to electrical conductivity changes showing an increasing reflection coefficient when electrical conductivity increases (e.g., in response to a salt tracer injection). Higher frequencies are more sensitive to fracture aperture variations showing an increasing reflection coefficient and phase shift

when aperture increases (e.g., in response to aperture widening induced by a high injection pressure).

Time-lapse GPR has proven effective when applied in fractured media with hydraulic transmissivities reaching  $10^{-3} - 10^{-5}$  m<sup>2</sup>/s corresponding to mm-scale fracture apertures (Dorn et al., 2011b; Shakas et al., 2016; Talley et al., 2005; Tsoflias & Becker, 2008) as well as in less permeable formations composed by sub-mm aperture fractures (Giertzuch et al., 2020). Typically, saline (i.e., electrically conductive) tracer injections at rates of L/min are performed in fractures with lower-salinity formation water (i.e., electrically less conductive). To our best know  $: _{a} - 10^{-9} - 10^{-10}$  m<sup>2</sup>/s).

This setting of very low hydraulic transmissivity is investig, ted in the context of long-term deep disposal of nuclear waste. Low permeable crystalline rocks at 400-600 m depth are or have been targeted for such repositories by countries such a. Sweden (Milnes, 2002), Finland (McEwen & Äikäs, 2000) and Canada (Davison et al., 1994), is they offer long-term mechanical, chemical and hydrogeological stability and could act as a potential barrier to leakage of contaminants (Neuman, 2005). In this contribution, we acquired une-lapse surface-based GPR data during a tracer test in a tunnel located at 410 m depth is the Äspö Hard Rock Laboratory, Sweden. In this experiment, aperture variations due to the hight water-injection pressure are expected (Rutqvist et al., 1998; Zang et al., 2016) with associate 1 impacts on the time-lapse GPR data.

Our experiment aimed to address the following questions:

- In very low-permeability fractured formations with apertures <0.1 mm, can time-lapse GPR data detect induced salinity or aperture variations?
- 2) Can GPR data be used to constrain hydromechanical models predicting aperture and permeability enhancement during high-pressure injection tests?
- 3) Is the time-lapse GPR method, when employed in such low-permeability environments, able to provide visual constraints on the connected fracture network between boreholes?

We performed 3-D GPR acquisitions before and at the end of the tracer test (sections 2.2 and 3.2). We then compared the observed time-lapse differences with GPR modeling for prescribed conductivity or aperture variations (section 4.1). We further predicted the expected fracture aperture enhancement given hydromechanical parameters and injection conditions (section 4.2).

### 2 Experimental set-up and data processing

The Äspö Hard Rock Laboratory (Sweden) is an underground research laboratory situated on the Simpevarp peninsula, Sweden, reaching 450 m below the sea level. It was constructed starting in 1990 (Cosma et al., 2001; Hammarström & Olsson, 1996) by the cived ish Nuclear Fuel and Waste Management Company (SKB). The laboratory has been us a to test and develop engineering methodologies. For instance, underground flow and transport characterization approaches have been developed (Selroos et al., 2002) that account for the associated chemical reactions to better constrain potential future radionuclide migration and reactions. Methodologies and procedures developed at Äspö Hard Rock Laboratory will subsequently be used in the development of the planned Swedish nuclear waste repositery (situated at Forsmark). Along the main tunnel (3.6 km long), several side-tunnels exist. The gec of y is mainly composed of fractured granitic rocks that are more than 1.7 billion years old (Cosme et al., 2001; SKB, 2016).

Our study tunnel, TASO4, is cituated at 410 m depth with a length of 36 m, a width of 4.2 m and a height of 5.3 m. The man, geological formations are composed by fine-grained granite, diorite and granodiorite. A 0.5 m thick portion of the original tunnel floor was cut and sawed by a diamond wire along 20 m to remove the fractures induced by the drill-and-blast excavation method (Ericsson et al., 2015; Ericsson et al., 2018). The resulting tunnel floor is flat and near horizontal leading to perfect conditions for the surface-based GPR method. This method was previously used to provide the most suitable siting of three 9.5 m deep boreholes (BH1, BH2 and BH3) crossing most sub-horizontal fractures imaged by the GPR (Molron et al., 2020). These boreholes were used for the hydraulic and tracer experiments (section 2.1).

#### 2.1 Hydraulic and tracer experiment

The hydraulic transmissivity of the fractured granite around TAS04 was initially estimated by hydraulic experiments performed by injection and outflow tests in 1-m packed-off sections in the three boreholes (BH1 to BH3) (Figure 1b). Only 5 of the 21 solicited sections provided flows above the flowmeter threshold (2 mL/min) despite imposed pressure differences of 1000-2000 kPa. The total hydraulic transmissivities are  $1.41 \times 10^{-9}$ ,  $2.2 \times 10^{-10}$  and  $7.0 \times 10^{-10}$  m<sup>2</sup>/s for BH1, BH2 and BH3 respectively. For more details, see Andersson and Ragvald (2021) and Molron et al. (2020).

The tracer test was performed November 6-7, 2018, between BH1 and BH2 using a convergentdipole geometry (higher outflow than injection rate) at high-p.oscure injection conditions. The outflow (BH1; 3.0-6.0 m depth) and injection (BH2; 3.2-3.7 m uepth) intervals were selected such that they correspond to the most transmissive zones in the boreholes. Double-packer systems were used to isolate them from other sections (Figure 1) T' $\epsilon$  injection device used is the EDZ-equipment (initially designed for injection tests in shallow both sholes to study the excavation damage zones) (Andersson & Ragvald, 2021), equipped by a data collector and control systems connected to water and nitrogen bottles, gathered on a t ol c' (Figure 1a). The experiment proceeded by imposing a constant high pressure that push s the tracer solution towards the packers via the connected hydraulic tubes, while precisely measuring the injection rate with a mass flow meter. Outflow from the recovery borehole w s established by opening the inter-packer section to the atmospheric pressure (no pump was us d). The electrical conductivity (EC) was continuously measured at the outflow location via an EC sensor connected to the data logger. The outflow was measured manually with a bucket as it was under the measurement limit of the flowmeter. The tracer solution was collected using a sampling machine equipped with a peristaltic pump that collected the outflowing solution at a constant flow into 19 mL tubes. The sampling period varied between 10 and 60 minutes during the tests. Given the saline formation water (EC  $\approx$  1.8 S/m) containing relict seawater, we used a resistive tracer (Uranine diluted in deionized water) to create an electrical contrast (lower EC) and, thereby, changes in GPR reflectivity. Due to the large transit time (two hours) of the tracer within the

plastic tubes connecting the EDZ device and the injection section, we pre-filled the tubes and the injection borehole with the tracer solution ( $C_0 = 0.97$  ppm). We then injected the solution during 25.6 hours with a mean injection pressure of 5000 kPa (starting with 4000 kPa for the 3.3 first hours) (Figure 2a) corresponding to a mean injected flow of 8.6 mL/min at steady state, implying a total injected volume of 13.3 L and an outflow of 20 mL/min.

(a)

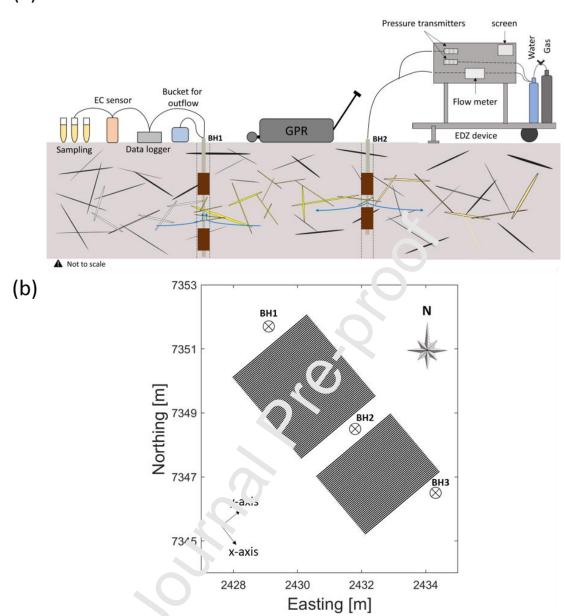


Figure 1: (a) schematic view of the experimental set-up (not to scale). A double packer system (in brown) is used to isolate the solicited borehole sections. The tracer solution is injected in BH2 by an EDZ-equipment with constant pressure and a flowmeter measuring the flow rate. The outflowing solution from BH1 is measured with an electrical conductivity (EC) sensor (connected to the data logger) and sampled. Surface-based GPR measurements were acquired before and during the tracer test. (b) 3-D GPR grid measurements in local Äspö96 coordinates in cross-line configuration following Molron et al. (2020) (along y-axis).

#### 2.2 GPR experiment

The 3-D GPR surveys were performed the day before (reference acquisition) and during the last hour (final acquisition) of the tracer experiment, in the period of November 5-7, 2018, using the MALÅ GroundExplorer (GX) HDR-series (High Dynamic Range). We used both 160 MHz and 450 MHz antennas to leverage their different sensitivities to the electrical conductivity of the fracture fluid and aperture (Tsoflias & Becker, 2008). The transmitter and receiver antennas are gathered in a shielded device that is pulled with a sledge on the cleaned and flat tunnel floor (surface-based method) using wooden planks and measurement tape to ensure straight parallel lines and a high degree of repeatability in terms of positioning between acquisitions. The sevaration between antennas is fixed (0.33 m and 0.18 m for 160 MHz and 450 MHz, respectively); a so-called common-offset configuration. The 3-D set-up geometry consists of man / parallel lines in "cross-line configuration" using the terminology of Molron et al. (2020) (along the y-axis corresponding to the tunnel width direction) between the chosen boreholes and be 'ond the injection hole (Figure 1b). A distance of 0.10 m and 0.05 m separated the profiles using 160 MHz and 450 MHz antennas, respectively. This resulted in 64 (160 MHz) and 129 (450 MHz), rofiles covering an area of 3 m x 6.3 m (including a 0.65 m wide region without measurem and oue to the injection borehole instrumentation). The timeperiods of the acquisitions with res, ect to the injection experiment are shown in Figure 2a.

In a previous study in the ame tunnel (Molron et al., 2020), the GPR data were processed to obtain images with magnitudes reflecting the spatially varying reflection coefficients of fractures. Furthermore, the processing workflow included a rather strong suppression of ringing events to avoid interpreting such artifacts as near-horizontal fractures. Based on the resulting images, it was then possible to identify the most prominent fracture reflections. In the present study, we are primarily interested in identifying the fractures showing the largest relative variations in reflectivity over time, as they are likely to correspond to changing aperture or electrical conductivity of the fluidfilled fractures in response to the injection experiment. Hydrological data (Molron et al., 2020) indicate further that near-horizontal fracture pathways are likely. Time-lapse comparisons of traces

make it easier to identify horizontal events than for static surveys as ringing effects are likely constant over time and can be removed by subtraction. Consequently, we decided to not remove horizontal events by applying, for example, singular value decomposition (SVD) when processing these data.

In the present study, GPR processing starts with an editing step that serves to prune additional traces of the GPR-profiles in order to obtain a rectangular measurement block. Direct current (DC) removal consists of removing data offsets and was here achieved by subtracting the median of the last 20 % of the unnormalized GPR traces. The time zero correction corrects the signal initiation time based on the antenna separation and the speed of light in a vacuum. This is achieved by shifting the time vector using an amplitude threshold chosen to be slightly above the noise level and to attribute the first such occurrence to the first-arrival of the airwave. Subsequently, a high-pass zero-phase filter is used to remove low frequencies below 80 and 160 '1H -, respectively. When performing time-lapse differencing, it is essential to overlay the tace, carefully with respect to signal attributes that are assumed to be time-invariant (in our case, the first 8.9 and 3.5 ns of the signal for 160 and 450 MHz, respectively) in order to ensure that tine lapse differences refer to actual in situ variations. For accurate trace alignment, we are imited by the finite sampling rate of the GPR system and perform, thus, upsampling using a Fourier transform to a ten times higher sampling rate. The resulting traces acquired at the same is ration at different times are aligned and the energy is normalized in the identified time-windows before downsampling the signal to the original sampling rate for further analysis. To ensure a similar magnitude range throughout the 3-D data volume, we perform automatic gain control (AGC) using the reference data and apply the resulting gain function to the final acquisitions as well. In this way, it is possible to assess relative differences over time as the gain function remains constant. This AGC processing allows us to identify regions with small reflectivity, but large relative temporal variations. This is helpful as it is not necessarily the regions with the highest GPR reflectivity that are stimulated by our experiments. The data volumes acquired at

different times are subtracted and differences observed are interpreted as being related to induced changes in the fractures or their fluids.

Lastly, we applied 3-D Kirchhoff migration (Margrave & Lamoureux, 2019) on the processed reference GPR acquisition data and the difference between processed GPR acquisitions (final - reference) to collapse the many diffractions and locate the GPR reflections at depth. This was achieved using the CREWES Matlab toolbox (CREWES, 1988) assuming a constant velocity of 0.125 m/ns, which provided the best collapse of the diffractions in the migrated images.

#### 3 Results

#### 3.1 Hydraulic data

The pressure conditions before and during the tracer test in depicted in Figure 2a. In BH1, the packed-off interval was open to the tunnel through the conflow tube, implying near-atmospheric conditions while the pressure was around 1830 kPc below the double packer. In BH2, the pressure well-before injection was around 1850 kPc on the injection section and around 2120 kPa below the double packer. At 1.5 hours before injection, the pressure in BH2 drops towards zero as the packer was momentarily deflated and filled with tracer solution. When injection started, the imposed pressure in BH2 was around 400° kPc for the first three hours after which it was raised to 5000 kPa. A resulting minor pressure increase is observed below the packers in BH1 and BH2 at levels of 1860 kPa and 2230 kPa, respectively. The short decrease 19.8 hours after the injection start in the injection interval was due to incorrect manipulation when closing the tracer bottle after the last refilling. The tracer recovery and its electrical conductivity are shown in Figure 2b. The first tracer arrival in BH1 is sampled after 1.95 hours of injection (after correcting for the transfer time of the tracer in the tubes). This time agrees with the time of the initial drop in electrical conductivity measured by the EC sensor.

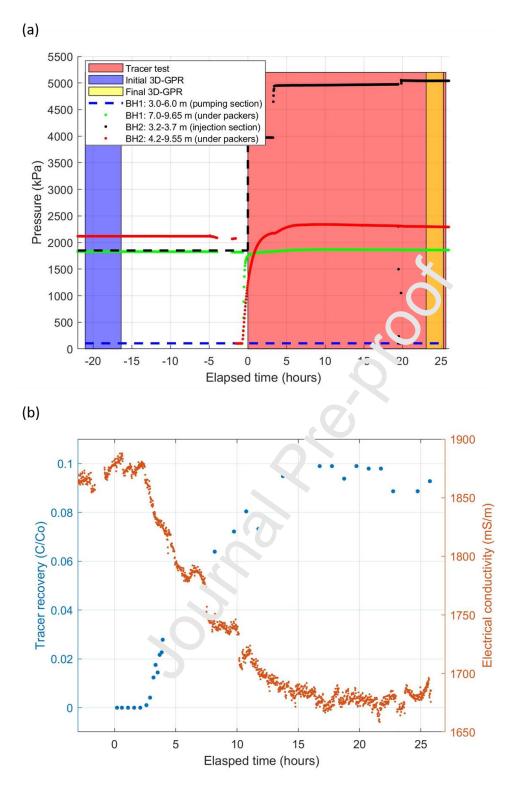


Figure 2: (a) Pressure data before and after the injection below the double packers in BH1 (green) and BH2 (red) and in the injection section (black). The recovery section (blue dashed lines; not measured) was open to the atmosphere (14 days before and during the tracer tests). The reference and final GPR acquisition times and the injection times are represented in blue, yellow and red, respectively. (b) Tracer recovery (blue) and electrical conductivity of the outflowing water at the exit of BH1 (orange).

#### 3.2 GPR data

Figure 3 presents horizontal visualizations of the processed 450 MHz 3-D GPR data for the reference acquisition (before the injection) (Figure 3a-c), the final acquisition (after injection) (Figure 3d-f), for data differences (final - reference) (Figure 3g-i) and migrated difference-data (Figure 3j-l). We selected three depths (2.5 m, 3.6 m and 4.3 m) corresponding respectively to one depth level above, one within and one below the injection interval. This was done to observe the behavior of the GPR signals at depths presumably affected (3.6 m) or unaffected (2.5 m and 4.3 m) by the tracer test. At 2.5 m depth (above the injection section), the structures feature a in the reference (Figure 3a) and final data (Figure 3d) disappear in the difference data and in the resulting difference migration results (Figure 3g,j). The very strong similarity between the revenue data (Figure 3a) and the final data (Figure 3d) at this depth interval suggests that high repetability in terms of positioning and the GPR processing procedure allow for highly comparable acquisitions. Indeed, even minute positioning or amplitude errors would have show up in by difference image (Figure 3g). At 3.6 m depth (within the injection section from 3.2 to 3.7 m), the structures resulting from the processed difference data (Figure 3h) correspond to diffraction-line events close to the boreholes. As we were unable to measure GPR data above the borcholes, it is difficult to establish the exact origin of these time-lapse differences. One possibility is a the diffractions associated with the metallic parts of the packers vary as the injection bore was filled by highly conductive formation water during the reference acquisition and by resistive tracer solution during the final acquisition.

After difference migration, the borehole-related diffractions have largely collapsed and leave features aligned along a linear path connecting the injection and extraction borehole (Figure 3k). Despite that most diffractions were collapsed, residual energy still remain around the injection borehole. This can be explained by the lack of measurements above the boreholes. At 4.3 m depth, only diffractions are observed on the unmigrated difference-data (Figure 3i) and they are mostly collapsed in the migrated difference-data (Figure 3I). By studying the migrated difference-data at

these depth intervals, we observe that coherent time-lapse differences away from the injection region appear only in the depth range at which we are injecting tracer and expect the tracer transport to take place.

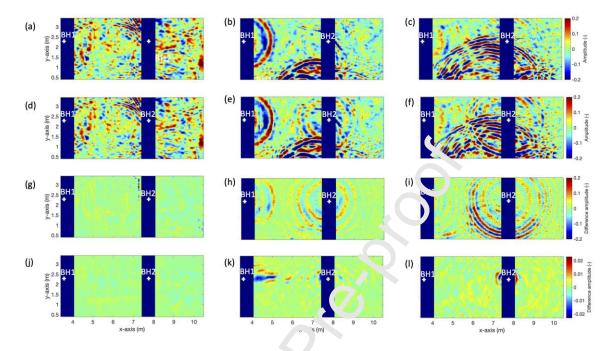


Figure 3: Depth slices of 3-D GPR measurements (450 Mr.  $^{1}$  at 2.5 m (a,d,g,j), 3.6 m (b,e,h,k) and 4.3 m (c,f,l,l) depth. (a,b,c,) the time-to-depth converted processed data before injection (reference), (d,e,f,) the time-to-depth converted processed data after injection (final), (g,h,i) the difference between the e-to-depth converted processed data (final - reference) and (j,k,l) the migrated difference-data (final - reference).

The 3-D migrated reference (Figure 4c) and difference-data (Figure 4b) were imported into the software Paradigm GOCAD<sup>TM</sup>. The Unline" (x-direction) vertical profile showed is an interpolation of the 129 cross-line profiles (y-c'irection) that were acquired. The term in-line is here used to be in agreement with Molron et al. (2020). The structures highlighted on Figure 3h are well visible at depths of 3.6 and 3.4 m between the solicited borehole sections (in yellow), and are well distinguished from the background signal.

Exemplary 3-D 160 MHz migration results are also given for the reference (Figure 5a) and difference data (Figure 5b). Compared to the 450 MHz data, we observe that more of the structures seen in the reference acquisition are left in the migrated difference-data, making data interpretation more challenging. The long patterns remaining after the differencing may suggest an origin from the surface. We used wooden planks to ensure straight parallel lines of the 3-D GPR measurements.

These planks were dry during the reference acquisition and wet during the final acquisition. Consequently, we cannot exclude different coupling between the antennas and the wooden planks that might have changed the effective wavelet in a way that we could not correct for with our processing scheme. Generally speaking, the 160 MHz data by Molron et al. (2020) had a much more "ringy" character than the 450 MHz data that was removed by an SVD-based processing step. However, no such step was applied here as we wanted to localize flow paths that were expected to be largely sub-horizontal. Despite these noisy structures, we observe similar patterns connecting BH1 and BH2 in the injection depth area (inter-packer section represented in yellow) (Figure 5b).

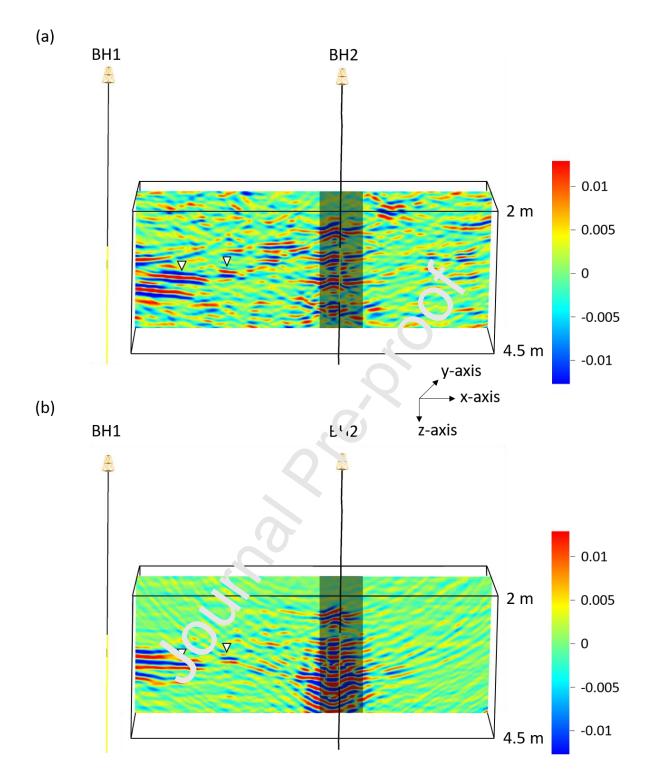


Figure 4: 3-D migrated 450 MHz data represented by a vertical slice passing through the chosen boreholes. (a) Migration of the processed reference GPR acquisition data (before injection) and (b) migration of difference-data (final – reference). The shaded area corresponds to the zone above which we did not perform GPR profile surveys. The injection and extraction boreholes are BH2 and BH1, respectively, with the solicited sections highlighted in yellow along the boreholes. The apexes of white triangles indicate the location of observed relative amplitude changes that are analyzed in section 4.1 (Figure 6c,d). The amplitudes are unitless and refer to the input data for migration that had normalized magnitudes varying between -1 and 1.

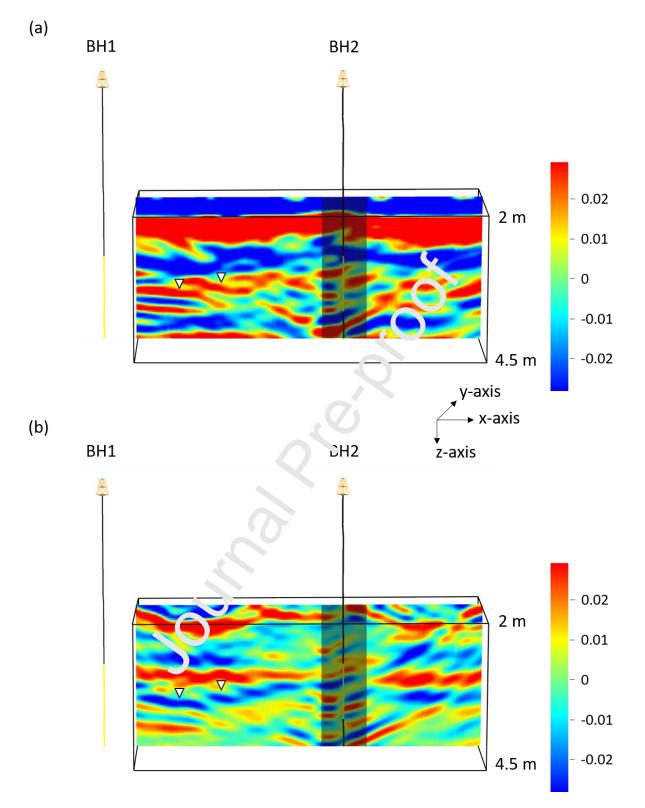


Figure 5: 3-D migrated 160 MHz data represented by a vertical slice passing through the chosen boreholes. (a) Migration of the processed reference GPR acquisition data (before injection) and (b) migration of differences between acquisition times (final - reference). The shaded area corresponds to the zone above which we did not perform GPR profile surveys. The injection and extraction boreholes are BH2 and BH1, respectively, with the solicited sections highlighted in yellow along the boreholes. The apexes of white triangles indicate the location of observed relative amplitude changes that are analyzed in section 4.1 (Figure 6a,b). The amplitudes are unitless and refer to the input data for migration that had normalized magnitudes varying between -1 and 1.

#### 4 Discussion

#### 4.1 GPR-based scenario modeling

Except for the area around the injection borehole (BH2) that is influenced by non-collapsed diffractions, we are able to identify fractures that are affected by the tracer injection on difference migrated 2-D profiles of 450 MHz data (Figure 4b). It is more difficult to observe these linear reflections on 160 MHz migrated difference-data (Figure 5b) due to the superimposed horizontal ringing patterns, but some coherent reflection trends are seen at the injection depth level. We analyzed the temporal variation of the amplitudes (Figure 6) and focus on two regions of temporal variations. These regions in-between the boreholes close to the injection depth (indicated in Figure 4 and Figure 5 by triangles) are considered to originate from two separate fractures. They are situated at 3.6 m (Figure 6a,c) and 3.4 m (Figure 6b,d) depth at 1.14 m and 2.18 m away from BH1 respectively. The time-lapse differences of these two migrated signals reveal an increase of the reflection magnitude for both frequencies, only at these depth regions.

To understand the possible origin of these tomporal variations, we modeled GPR signals from a fracture for varying electrical conductive we and fracture aperture. To do so, we used the semianalytical, frequency-domain-based, ensective-dipole forward modeling approach by Shakas and Linde (2015). In this approach, a fracture of prescribed size and orientation (dip and strike) is discretized by squares at the second rescaled times smaller than the dominant wavelength. The source radiation in a unifor n isotropic rock matrix with prescribed electrical properties is calculated analytically, assuming an infinitesimal dipole and a prescribed source wavelet, from which the tangential component of the electrical field is recorded at the center of each discretized element. Using this information, the area of the discretized element, and assumed fracture properties (electrical conductivity and permittivity of the fracture filling) together with the thin-bed solution, it is possible to specify a corresponding electrical dipole at each discretized element of the fracture surface. Subsequently, each such secondary dipole is transmitted to the receiver location using the same analytical solution used for source radiation modeling. By summing the contributions of all

secondary dipoles over the discretized fracture at the receiver location, it is possible to simulate the GPR response of fractures with heterogeneous electrical conductivity and permittivity fields very cheaply at high accuracy (Shakas and Linde, 2015; Shakas and Linde, 2017). Here, we consider a square and homogeneous horizontal fracture with dimension of 2 m situated at 3.6 m depth (similar as the first GPR data reflection analyzed in Figure 6a,c). The rock matrix is given an electrical conductivity of 2 x 10<sup>-4</sup> S/m and relative permittivity of 6. The EC of the fracture-fluid was chosen as 1.8 S/m (corresponding to the saline formation water) and the initial aperture was fixed at 5.7 x 10<sup>-5</sup> m (considering a mechanical fracture aperture reaching 8.5 x 10<sup>-5</sup> m, calculated during tracer test, and a 50% widening induced by the high-pressure injection: detailed calculations are shown in supplementary material). The antenna dipole moments are modelled as a generalized gamma distribution [Shakas and Linde, 2015] with frequency spectra that are similar to the observed frequency contents of the 160 MHz and 450 MHz GI R an tennas, respectively.

First, we assessed how the GPR signal re nords when the EC of the fracture-fluid is reduced in response to the injection of the resistive cracer. To do so, we considered a decrease to EC = 1.0 S/m, which is much lower than the value observed at peak tracer breakthrough of 1.6 S/m in BH1. This is done because dilution in BH1 is endeded as it draws water from several fractures, with some of them not being involved in the tracer transport. The modeled 450 MHz (Figure 7a) and 160 MHz (Figure 7c) traces show an amplitude decrease of 25% and 35% respectively. Next, we consider the impact of an increase in fracture aperture that might arise in response to high-pressure injection while the electrical conductivity of the formation water was kept fixed at 1.6 S/m. Several tests were made with all leading to amplitude increases. The modeling results for 450 MHz (Figure 7b) and 160 MHz (Figure 7d) are given for an aperture increase of 50% leading to corresponding increases of 49% in terms of both peak amplitudes (observed for the first positive peak). By comparing with the actual data in Figure 6, we see that the chosen scenario of a 50% aperture increase provides similar responses to those being observed. Thus, the observed increases of GPR reflectivity in the stimulated fractures suggest that the observed GPR time-lapse response is dominated by aperture increases due

to high injection pressure rather than electrical conductivity decreases due to the injected tracer that tend to decrease reflectivity. Since the tracer injection is expected to decrease the electrical conductivity of the fracture-fluid somewhat, we expect that a slightly lower increase in reflectivity would occur than the one considered in the scenario of an increasing aperture and constant fracturefluid properties. However, given the important uncertainties involved in the modeling, we refrain from considering a mixed scenario involving increasing aperture and decreasing salinity. We stress that the amplitudes obtained by the difference migrations (Figure 6) and the modelling results (Figure 7) are not comparable, while the relative variations in amp."tudes are. Indeed, the actual magnitude of the source is unknown and can, thus, not be modeled in the next section, we assess if the suggested 50% increase in fracture aperture during tracer injection is consistent with hydromechanical considerations.

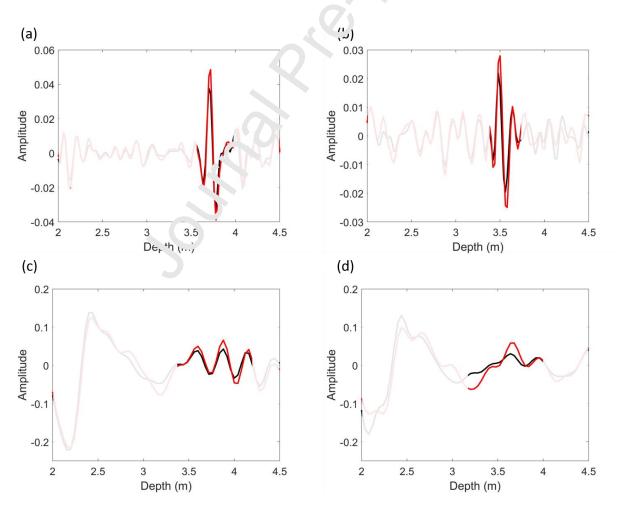
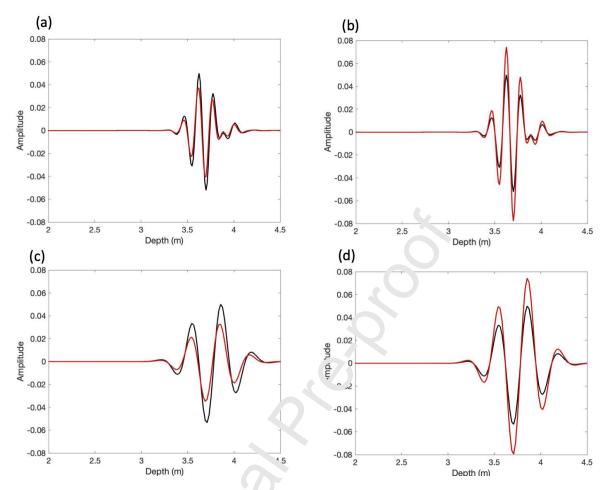


Figure 6: 3-D Field migrated trace data for (a,b) the 450 MHz and (c,d) the 160 MHz data. Large GPR reflections originated from fractures situated (a,c) at 3.6 m of depth and at 1.14 m away from BH1 and (b,d) at 3.4 m of depth and at 2.18 m away from BH1. These reflections are pointed by white triangles on 2-D vertical slices on Figure 4 (450 MHz) and Figure 5



(160 MHz). The signals from reference and final GPR acquisitions are represented in black and red respectively. The amplitudes are unitless and refer to the input data for migration that had normalized magnitudes varying between -1 and 1.

Figure 7: GPR trace modeling for (a,b) 450 M<sup>++-</sup> fre<sup>++</sup> ency and (c,d) 160 MHz frequency. (a,c) We imposed a decrease of fluid EC filling the fracture with reference trace black, corresponding to the initial EC at 1.8 S/m (from saline water table) and final trace (red) corresponding to the E<sup>+</sup> at 1.7 S/m (from tracer solution filling BH2 just before injection start); (b,d) we imposed a 50% increase of fracture widen. To with reference trace (black) corresponding to an aperture of 57  $\mu$ m and final trace (red) corresponding to an aperture of 55  $\mu$ m. The focus on the interpretation and the comparison with Figure 6 should be based on relative magnitude variation<sup>+</sup>, the magnitudes are not comparable as the true source magnitude is unknown.

4.2 Is the hypothesis of a 50% aperture increase compatible with hydromechanics? The hypothesis that fractures may open up unevenly due to pressure increases in a 450 m deep Äspö tunnel section was investigated by Fransson et al. (2010) based on hydraulic testing and grouting records. The results obtained were less conclusive than for much shallower tunnels (Fransson et al., 2010) and they provide no estimate about aperture variations. In this section, we analyze the present tracer experiment and test whether the 50% increase in aperture suggested by the GPR analysis is consistent with the predicted aperture widening given the pressure change induced by pumping, considering what is known about the hydromechanical behavior of fractured rocks.

Before the injection test, when the first GPR survey was percended, BH1 was at atmospheric pressure and BH2 at ~1800 kPa (Figure 2a). During the infection test, the fluid pressure in BH2 increases up to 5000 kPa with no change in BH1. Figure a shows the differential fluid pressure that is assumed to increase linearly between the GPR surveys in the line joining BH1 and BH2 at the injection depth. The total pressure between and fracture walls is expected to decrease by the same value, entailing an increase of the fracture aperture. In the locations where the GPR signals from fractures were analyzed (pointed by whith apexes on Figure 4 and Figure 5), we estimate the decrease of wall pressure (Figure C) to be 870 kPa (for the fracture situated at 1.14 m away from BH1; Figures 6a,c) and 1700 kPa Conthe fracture situated at 2.18 m away from BH1; Figures 6b,d).

The variation of fracture aperture due to pressure change can be predicted either from the rock compressibility or the fracture stiffness. Rock compressibility of granitic or hardrock geological formations is commonly very low when measured on small unfractured (or microfractured) laboratory samples, generally of the order of 1/(10 GPa) at ambient pressure down to 1/(100 GPA) at high pressure (Brace, 1965). At pressures below 100 MPa, fractures play a dominant role in the compressibility of hard rocks and most of the rock deformation is concentrated in or around fractures (Walsh, 1965; Walsh & Grosenbaugh, 1979). By assuming that most of the deformation results from a change in aperture, a change of the rock mass volume V is  $\Delta V = \Delta \bar{a} p_{32} V$ , where  $p_{32}$  is the total surface of fractures per unit volume and  $\Delta \bar{a}$  the variation of the average fracture aperture

over the fracture network. Furthermore,  $p_{32}$  is related to the average aperture  $\bar{a}$  and to the total rockmass porosity  $\phi$  by  $\phi = \bar{a} p_{32}$ . The average aperture variation can then be calculated from:

$$\frac{\Delta \bar{a}}{\bar{a}} = \frac{1}{\phi} \frac{\Delta V}{V} = \frac{\beta_r}{\phi} \Delta P,$$
<sup>(1)</sup>

with  $\beta_r$  the bulk rock mass compressibility at the scale of interest. The total porosity for Äspö rocks has been measured to be around 0.2-0.3% (Autio et al., 2003) and  $\beta_r$  estimated for rock samples with microcracks at about  $1/(40 \pm 10 \text{ GPa})$  as derived from the bulk modulus given by Hakami et al. (2008). This results in a ratio  $\frac{\beta_r}{\phi}$  of  $1/(120 \pm 40 \text{ MPa})$  resulting in 1 change that is less than 1 % for a pressure increase of 1 MPa, that is, much less than the value escimated from the GPR modelling. The problem with the preceding analysis is that the elastic constants are not measured at the right scale, and not for the type of fractures that are imaged by GPR. For fractured rocks, we expect the Young and bulk modulus to decrease when increasing the rock mass scale, entailing an increase in compressibility (Davy et al., 2018). Values of compressibility as low as 1/(5 GPa) have been reported by Zangerl et al. (2008b) to explain the standard compressibility as low as 1/(5 GPa) have been reported is compatible with that predicted by the GPR data.

Another approach in estimate the aperture variation can be done from the normal fracture stiffness  $k_n$ :

$$\frac{\Delta a}{a} = \frac{\Delta P}{k_n a}.$$
(2)

This requires estimating both the initial aperture a and the normal stiffness  $k_n$ . The former can be deduced from hydraulic testing, either by converting transmissivity values into equivalent "hydraulic" aperture through the cubic law (Tsang & Witherspoon, 1981; Witherspoon et al., 1980), or by inferring it from tracer tests (derived from Thiem's solution (Thiem, 1906)). The hydraulic aperture was estimated at about 12  $\mu$ m and the mechanical aperture from tracer at about 85  $\mu$ m (see

supplementary material), the latter being the most appropriate estimate of mechanical aperture (Tsang, 1992). Estimating the normal stiffness is challenging since  $k_n$  is not an intrinsic property of the material but depends on the effective normal stress and likely also on the fracture size. Direct measurements from grouting data gives values of 35 GPa/m and 600 GPa/m for two boreholes at 450 m depth in the Äspö tunnel (Fransson et al., 2010). The smaller value for  $k_n$  gives an aperture variation of ~35% for a pressure change of 1 MPa. Other estimates of  $k_n$  have been reviewed by Zangerl et al. (2008a) by using the semi-logarithmic closure law between aperture and effective stress  $\sigma'_n$ ,  $-\Delta a = a(\sigma'_o) - a(\sigma'_n) = a_c \ln\left(\frac{\sigma'_n}{\sigma'_o}\right)$ , developed by Walsh and Grosenbaugh (1979) for the closure of surfaces whose topography is characterized by a responential distribution of summit asperity heights. Zangerl et al. (2008a) compiled values of the 'stiffness characteristic' ( $a_c^{-1}$ ), which leads to  $a_c$  ranging from 10 µm to 100 µm for granite samples of different sizes (up to 3 m) and corresponding relative fracture variations of 20-2C05.

Considering the uncertainty on hydromech nical parameters, we conclude that the aperture variation inferred by GPR is not inconsistent, with hydromechanical parameters. Larger scale *in situ* experiments as those presented herein, for GPR is a good way forward to obtain better estimates of in-situ elastic parameters at the scale of interest (e.g., Zangerl et al. (2008a) and Zangerl et al. (2008b)).

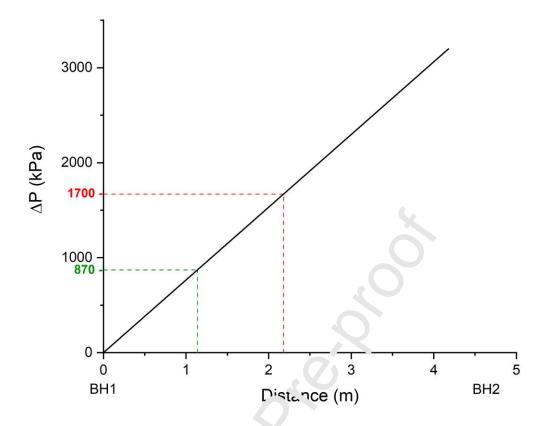


Figure 8: Pressure differences between boreholes 1.741 and BH2) between reference and final GPR acquisitions. The pressure differences indicated in green and red are situated at 1. analyzed GPR signals located 1.14 and 2.18 m away from BH1.

### 5 Conclusion

GPR data acquired during high-pressure injection of a resistive tracer were used to image fracture connectivity and induce aperture variations between two boreholes (BH1 and BH2). The experiment was performed in fractured rock of very low permeability a few meters beneath a tunnel situated at 410 m depth. We injected the tracer solution (Uranine + deionized water) with constant highpressure in a convergent-dipole geometry. To image the tracer pathways, we performed 3-D surfacebased GPR. GPR reflections in very low permeable environments with sub-mm fracture aperture are weak. Nevertheless, the difference of the final with respect to the efference GPR acquisition data provides a response that is consistent with the tracer injection *lepth*, particularly for the higherquality 450 MHz data that only show difference signals related to the depth interval of the tracer injection. The observed 45% increase in fracture reflect vity is reproduced by GPR-modeling when considering a 50% increase in fracture aperture, while a decrease in salinity due to the tracer injection leads to a smaller decrease in reflectivity (25%). Considering the large uncertainty in hydromechanical parameters at the site, we find that a 50% increase in aperture in response to the injection pressure is compatible with hyar mechanical considerations. This suggests that information on aperture variations provided by field scale in situ GPR experiments during hydraulic stimulation could help to better estimate platic parameters of the studied fractured rock. Given the low injection rate of 8.6 mL/m<sup>-</sup>n, ve were unable to significantly change the electrical properties of the fluid-filled fractures in this 1 rmation of very low permeability (hydraulic fracture aperture of 12  $\mu$ m). This limits the practical applicability of time-lapse GPR-assisted tracer-monitoring in the development of nuclear waste repositories. Our injection experiment also highlights challenges when high injection pressures are needed to observe clear measurement responses, as the induced fracture widening leads to an upward bias on transmissivity estimates with associated implications for risk assessments.

### Acknowledgements

ENIGMA ITN project that has received funding from the European Union's Horizon 2020 research and innovation programme under the Marie Sklodowska-Curie Grant Agreement No <u>722028</u>. Furthermore, we received significant financial support from SKB. The data used for this study are stored in the Sicada database and are available by an official request to the company. We would like to thank Lars Andersson, Patrik Vidstrand and Siren Bortelid Robertsson (SKB) and Peter Andersson and Johanna Ragvald (Geosigma) for their technical support and quality fieldwork. We also thank Diego Mas Ivars (SKB) for important input in terms of hydromechance, Christian Le Carlier De Veslud (Géosciences Rennes) for his support with the use of Paradigm GO AD <sup>M</sup> and Silvia De Simone for her help with the different aperture estimations. This study is also cupported by the French National Observatory H+. The GPR and tracer tests data are available at http://hplus.ore.fr/en/molron-et-al-2021-eg-data and, after a requesting login through in p://hplus.ore.fr/en/database/acces-database. Constructive reviews from two anonymous reviewers and Editor Wasowski helped to significantly improve our manuscript.

## Bibliography

- Andersson, P., & Ragvald, J. (2021). ENIGMA Project GPR monitoring of fractures at Äspö HRL. Results of field investigations in TAS04, SKB Report P-19-17, Svenk Kärnbränslehantering AB.
- Autio, J., Hjerpe, T., & Siitari-Kauppi, M. K. (2003). *Porosity, diffusivity and permeability of EDZ in crystalline rock and effect on the migration in a KBS-3 type repository.* Paper presented at the CLUSTER conference held in Luxembourg on 3-5 november 2003.
- Becker, M., & Tsoflias, G. (2010). Comparing flux-averaged and resident concentration in a fractured bedrock using ground penetrating radar. *Water Resour. Res.*, 46(9). doi: https://doi.org/10.1029/2009WR008260
- Brace, W. F. (1965). Some new measurements of linear compressibility of rocks. J. Geophys. Res., 70(2), 391-398. doi:https://doi.org/10.1029/JZ070i002p00391
- Cosma, C., Olsson, O., Keskinen, J., & Heikkinen, P. (2001). Seismic characterization of fracturing at the Äspö Hard Rock Laboratory, Sweden, from the kilometer scale to the meter scale. *Int. J. Rock Mech. Min.*, *38*(6), 859-865. doi: https://doi.org/10.1017/S1365-1609(01)00051-X
- CREWES. (1988). CREWES Matlab Toolbox. https://www.crewes.org/?escarchLinks/FreeSoftware/. (accessed 20 November 2017)
- Davison, C. C., Brown, A., Everitt, R. A., Gascoyne, M., Kozak, E. L., Endha, G. S., Martin, C. D., Soonawala, N. M., Stevenson, D. R., & Thorne, G. A. (1954) *The disposal of Canada's nuclear fuel waste: Site screening and site evaluation technology* (AECL-10719, COG-93-3). Retrieved from Whiteshell Laboratories, Pinawa, MB, Canaca
- Davy, P., Darcel, C., Le Goc, R., & Mas Ivars, D. (2018). Elactic properties of fractured rock masses with frictional properties and power law fracture size distributions. *J. Geophys. Res.: Solid Earth*, 123(8), 6521-6539. doi:https://doi.org/10/1029/2017JB015329
- Day-Lewis, F. D., Lane, J. W., Harris, J. M., & Genetic's S. M. (2003). Time-lapse imaging of salinetracer transport in fractured rock using difference-attenuation radar tomography. *Water Resour. Res., 39*(10). doi: https://doi.org/10.1029/2002wr001722
- Day-Lewis, F. D., Slater, L. D., Robinson, J., 'ohnson, C. D., Terry, N., & Werkema, D. (2017). An overview of geophysical techno'o, 'es appropriate for characterization and monitoring at fractured-rock sites. *J. Environ. N ar age., 204*, 709-720. doi:https://doi.org/10.1016 j.je. vman.2017.04.033
- Dorn, C., Linde, N., Doetsch, J., Le Borbne, T., & Bour, O. (2011a). Fracture imaging within a granitic rock aquifer using multiple-offset single-hole and cross-hole GPR reflection data. J. Appl. *Geophys.*, 78, 123-132. doi nttps://doi.org/10.1016/j.jappgeo.2011.01.010
- Dorn, C., Linde, N., Le Borg ie, C., Bour, O., & Baron, L. (2011b). Single-hole GPR reflection imaging of solute transport in a granitic aquifer. *Geophys. Res. Lett., 38*(8). doi: https://doi.org/10.1029/2011gl047152
- Ericsson, L., R, C., Lehtimäki, T., Ittner, H., Hansson, K., Butron, C., Sigurdsson, O., & Kinnbom, P. (2015). *A demonstration project on controlling and verifying the excavation-damaged zone*, (R-14-30). Retrieved from Stockholm, Sweden
- Ericsson, L., Vidstrand, P., Christiansson, R., & Morosini, M. (2018). *Comparison Between Blasting and Wire Sawing Regarding Hydraulic Properties of the Excavated Damaged Zone in a Tunnel– Experiences From Crystalline Rock at the Äspö Hard Rock Laboratory, Sweden.* Paper presented at the 52nd US Rock Mechanics/Geomechanics Symposium.
- Fransson, Å., Tsang, C.-F., Rutqvist, J., & Gustafson, G. (2010). Estimation of deformation and stiffness of fractures close to tunnels using data from single-hole hydraulic testing and grouting. *Int. J. Rock Mech. Min., 47*(6), 887-893. doi:https://doi.org/10.1016/j.ijrmms.2010.05.007
- Giertzuch, P.-L., Doetsch, J., Jalali, M., Shakas, A., Schmelzbach, C., & Maurer, H. (2020). Time-lapse ground penetrating radar difference reflection imaging of saline tracer flow in fractured rock. *Geophysics, 85*(3), H25-H37. doi:https://doi.org/10.1190/geo2019-0481.1
- Gudmundsson, A. (2011). *Rock fractures in geological processes*. Cambridge University, New York: Cambridge University Press.

- Hakami, E., Fredriksson, A., Lanaro, F., & Wrafter, J. (2008). *Rock mechanics Laxemar: site descriptive modelling SDM-Site Laxemar* (R-08-57). Retrieved from Stockholm, Sweden
- Hammarström, M., & Olsson, O. (1996). *Äspö Hard Rock Laboratory. 10 years of research*: Svensk Kärnbränslehantering AB, Sweden.
- Klepikova, M. V., Le Borgne, T., Bour, O., Dentz, M., Hochreutener, R., & Lavenant, N. (2016). Heat as a tracer for understanding transport processes in fractured media: Theory and field assessment from multiscale thermal push-pull tracer tests. *Water Resour. Res., 52*(7), 5442-5457. doi: https://doi.org/10.1002/2016WR018789
- Margrave, G. F., & Lamoureux, M. P. (2019). *Numerical Methods of Exploration Seismology: With Algorithms in MATLAB®*: Cambridge University Press.
- McEwen, T., & Äikäs, T. (2000). *The site selection process for a spent fuel repository in Finland. Summary report* (POSIVA 2000-15). Retrieved from Helsinki, Finland
- Milnes, A. G. (2002). Swedish deep repository siting programme. Guide to the documentation of 25 years of geoscientific research (1976-2000), (TR-02-18). Retrieved from Stockholm, Sweden
- Molron, J., Linde, N., Baron, L., Selroos, J.-O., Darcel, C., & Davy, P. (2029). Which fractures are imaged with Ground Penetrating Radar? Results from an experiment in the Äspö Hardrock Laboratory, Sweden. *Eng. Geol., 273*. doi:https://doi.org/10.1016/j.enggeo.2020.105674
- National Research Council. (1996). *Rock fractures and fluid flow · cor temporary understanding and applications*: National Academies Press.
- Neuman, S. P. (2005). Trends, prospects and challenges in quantifying flow and transport through fractured rocks. *Hydrogeol J, 13*(1), 124-147. doi:n:ths.//doi.org/10.1007/s10040-004-0397-2
- Rutqvist, J., Noorishad, J., Tsang, C. F., & Stephansson. O. (1992). Determination of fracture storativity in hard rocks using high-pressure injection testing. *Water Resour. Res., 34*(10), 2551-2560. doi:https://doi.org/10.1029/23Wh01863
- Selroos, J. O., Walker, D. D., Ström, A., Gyllin, B. & Ollin, S. (2002). Comparison of alternative modelling approaches for groundwaler flow in fractured rock. *J. Hydrol., 257*(1-4), 174-188. doi: https://doi.org/10.1016/S0022-1694(01)00551-0
- Shakas, A., & Linde, N. (2015). Effective modeling of ground penetrating radar in fractured media using analytic solutions for propaga ion, thin-bed interaction and dipolar scattering. J. Appl. Geophys., 116, 206-214. doi: http://doi.org/10.1016/j.jappgeo.2015.03.018
- Shakas, A., Linde, N., Baron, L., Boci, et, O., Bour, O., & Le Borgne, T. (2016). Hydrogeophysical characterization of transport processes in fractured rock by combining push-pull and single-hole ground penetrating rodar experiments. *Water Resour. Res., 52*(2), 938-953. doi: https://doi.org/10.1002/2015WR017837
- Shakas, A., Maurer, H., Gie tzuci, P. L., Hertrich, M., Giardini, D., Serbetto, F., & Meier, P. (2020). Permeability enhancement from a hydraulic stimulation imaged with Ground Penetrating Radar. *Geophys. Rev. Lett., 47*(17). doi:https://doi.org/10.1029/2020GL088783
- SKB. (2016). *Äspö Hard Rock Laboratory Annual Report 2015* (TR-16-10). Retrieved from Stockholm, Sweden
- Talley, J., Baker, G. S., Becker, M. W., & Beyrle, N. (2005). Four dimensional mapping of tracer channelization in subhorizontal bedrock fractures using surface ground penetrating radar. *Geophys. Res. Lett.*, 32(4). doi: https://doi.org/10.1029/2004GL021974
- Thiem, G. (1906). Hydrologische Methoden, Gebhardt, Leipzig. In: Germany.
- Tsang, C. F., & Neretnieks, I. (1998). Flow channeling in heterogeneous fractured rocks. *Rev. Geophys.*, *36*(2), 275-298. doi:https://doi.org/10.1029/97RG03319
- Tsang, Y. (1992). Usage of "equivalent apertures" for rock fractures as derived from hydraulic and tracer tests. *Water Resour. Res., 28*(5), 1451-1455. doi: https://doi.org/10.1029/92WR00361
- Tsang, Y., & Tsang, C. (1989). Flow channeling in a single fracture as a two-dimensional strongly heterogeneous permeable medium. *Water Resour. Res., 25*(9), 2076-2080. doi:https://doi.org/10.1029/WR025i009p02076

- Tsang, Y. W., & Witherspoon, P. (1981). Hydromechanical behavior of a deformable rock fracture subject to normal stress. *J. Geophys. Res.: Solid Earth, 86*(B10), 9287-9298. doi:https://doi.org/10.1029/JB086iB10p09287
- Tsoflias, G. P., & Becker, M. W. (2008). Ground-penetrating-radar response to fracture-fluid salinity: Why lower frequencies are favorable for resolving salinity changes. *Geophysics*, 73(5), J25-J30. doi: https://doi.org/10.1190/1.2957893
- Tsoflias, G. P., Halihan, T., & Sharp, J. M. (2001). Monitoring pumping test response in a fractured aquifer using ground-penetrating radar. *Water Resour. Res., 37*(5), 1221-1229. doi: https://doi.org/10.1029/2000wr900297
- Walsh, J. (1965). The effect of cracks on the compressibility of rock. J. Geophys. Res., 70(2), 381-389. doi:https://doi.org/10.1029/JZ070i002p00381
- Walsh, J., & Grosenbaugh, M. (1979). A new model for analyzing the effect of fractures on compressibility. *J. Geophys. Res.: Solid Earth, 84*(B7), 3532-3536. doi:https://doi.org/10.1029/JB084iB07p03532
- Witherspoon, P. A., Wang, J. S., Iwai, K., & Gale, J. E. (1980). Validity of oubic law for fluid flow in a deformable rock fracture. *Water Resour. Res.*, *16*(6), 1016-102-1. doi:https://doi.org/10.1029/WR016i006p01016
- Zang, A., Stephansson, O., Stenberg, L., Plenkers, K., Specht, S., Milk :reit, C., Schill, E., Kwiatek, G., Dresen, G., Zimmermann, G., Dahm, T., & Weber, M. '2016). Hydraulic fracture monitoring in hard rock at 410 m depth with an advanced fluid-injection protocol and extensive sensor array. *Geophys. J. Int., 208*(2), 790-813. doi: https.//dri.org/10.1093/gji/ggw430
- Zangerl, C., Eberhardt, E., Evans, K., & Loew, S. (2008a). Consolidation settlements above deep tunnels in fractured crystalline rock: Part 2- Nur verical analysis of the Gotthard highway tunnel case study. *Int. J. Rock Mech. Min.*, 45(c), 1211-1225. doi:https://doi.org/10.1016/j.ijrmms 20, 8.02.005
- Zangerl, C., Evans, K., Eberhardt, E., & Loew, 2 (2008b). Consolidation settlements above deep tunnels in fractured crystalline rock: Pan 1—Investigations above the Gotthard highway tunnel. *Int. J. Rock Mech. Min.,* 45(ح), 1195-1210. doi:https://doi.org/10.1016/j.ij m.n.:2008.02.002

Justine Molron: Conceptualization, Methodology, Investigation, Formal analysis, Writing - Original Draft, Writing - Review & Editing, Data curation, Visualisation, Methodology, Software Niklas Linde.: Conceptualization, Methodology, Investigation, Formal analysis, Supervision, Writing - Original Draft, Writing - Review & Editing. Philippe Davy: Conceptualization, Methodology, Formal analysis, Supervision, Writing - Original Draft Investigation, Writing - Review & Editing Ludovic Baron: Methodology, Investigation: Caroline Darcel: Conceptualization, Methodology, Supervision, Writing -Review & Editing: Jan-Olof Selroos: Conceptualization, Methodology, Writing - Review & Editing.: Tanguy Le Borgne: Writing - Review & Editing, Investigation: Diane Doolaeghe: Investigation

### **Declaration of interests**

 $\boxtimes$  The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

□The authors declare the following financial interests/personal relationships which may be considered as potential competing interests:

### Abstract

We assess the performance of the Ground Penetrating Radar (GPR) method in fractured rock formations of very low transmissivity (e.g.  $T \approx 10^{-9}$ -  $10^{-10}$  m<sup>2</sup>/s for sub-mm apertures) and, more specifically, to image fracture widening induced by high-pressure injections. A field-scale experiment was conducted at the Äspö Hard Rock Laboratory (Sweden) in a tunnel situated at 410 m depth. The tracer test was performed within the most transmissive sections of two boreholes separated by 4.2 m. The electrically resistive tracer solution composed of deionized water and Uranine was expected to lead to decreasing GPR reflections with respect to the saline *in six* formation water. The injection pressure was 5000 kPa leading to an injection rate of 8.6 m /mi (at steady state) that was maintained during 25 hours, which resulted in a total injected volume of 13 L. To evaluate the fracture pathways between the boreholes, we conducte<sup>-</sup> 3-D surface-based GPR surveys before and at the end of the tracer tests, using 160 MHz and 450 MHz antennas. Difference GPR data between the two acquisitions highlight an increasing frontune reflectivity in-between the boreholes at depths corresponding to the injection interval. GPR-b. sed modeling suggests that the observed increasing reflectivity is not due to the tracer solution but rather to a 50% widening of the fracture. Considering prevailing uncertainties in material properties, a hydromechanical analysis suggests that such a degree of widening is feasible. This research demonstrates that field-scale in situ GPR experiments may provide constraints or, tra-ture widening by high-pressure injection and could help to constrain field-scale elastic parameters in fractured rock.

**Key words:** Ground penetrating radar, Surface-based method, Fracture, Tunnel, Nuclear waste disposal, High-pressure injection, Tracer test, Hydromechanics

- Time-lapse GPR investigation in the context of nuclear waste repositories
- High-pressure injection in granitic sub-millimeter fractures at 410 m depth
- GPR temporal variations originate from hydromechanically-induced fracture widening
- GPR modeling suggests a 50% increase of fracture aperture
- The inferred aperture increase is compatible with hydromechanical modeling

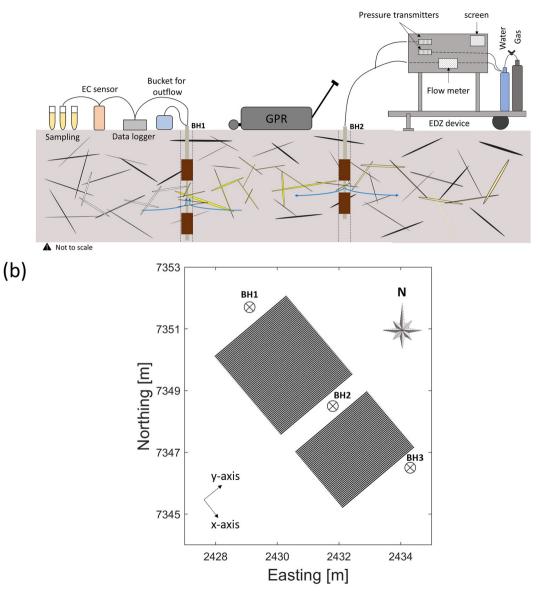
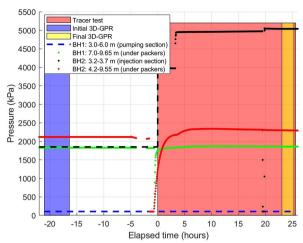


Figure 1





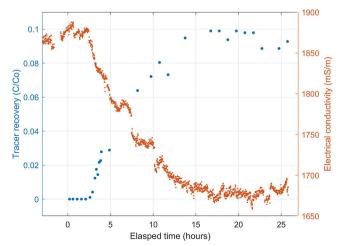
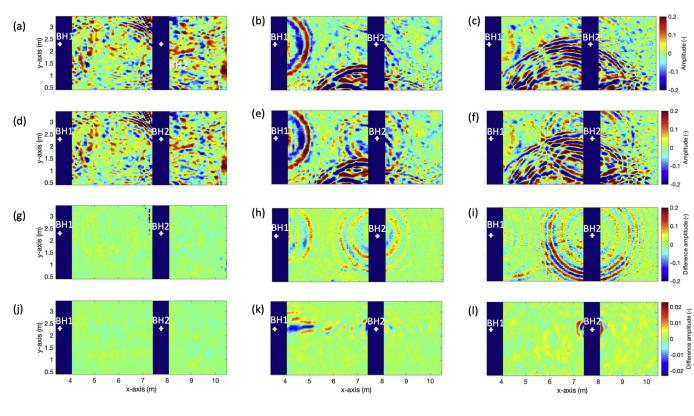


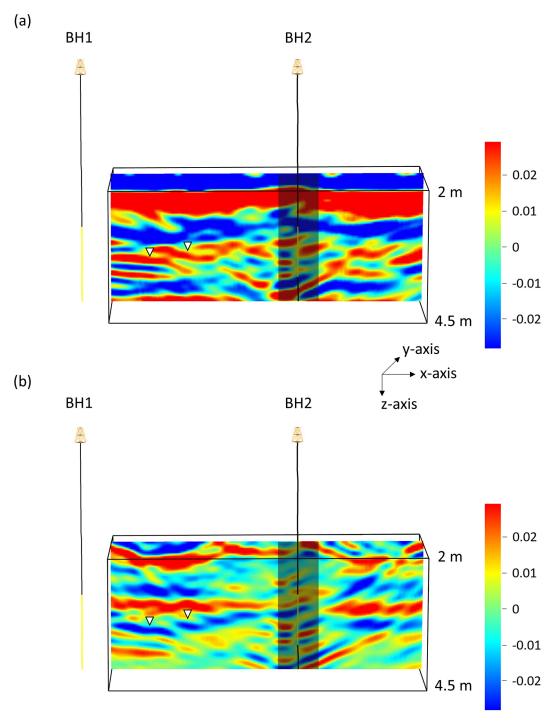
Figure 2

(a)



(a) BH1 BH2 - 0.01 2 m - 0.005 - 0 - -0.005 4.5 m -0.01 y-axis ⋆ x-axis (b) , z-axis BH1 BH2 - 0.01 2 m - 0.005 - 0 - -0.005 - -0.01 4.5 m

Figure 4



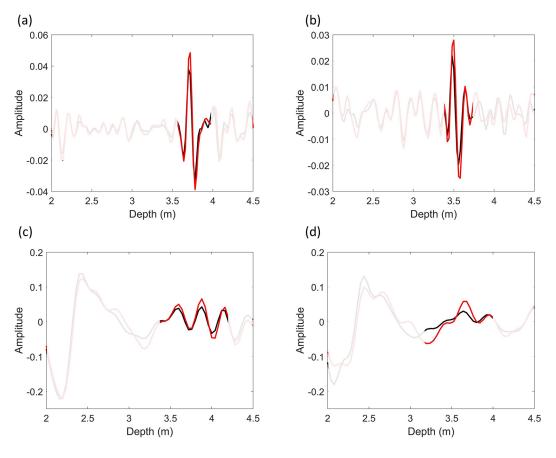


Figure 6

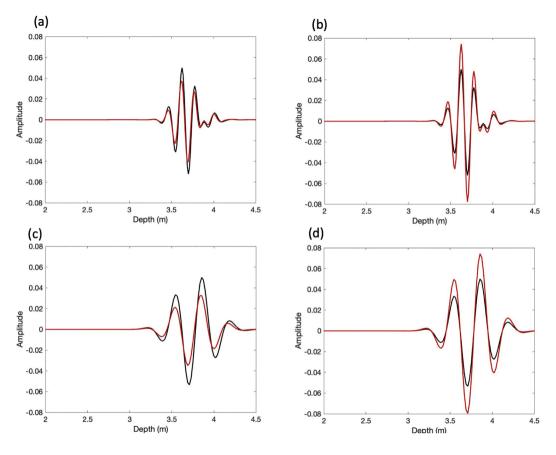


Figure 7

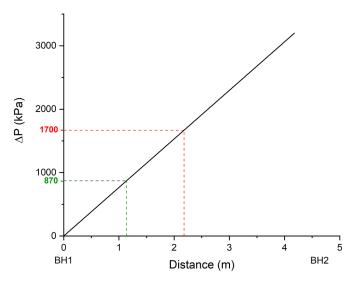


Figure 8